

PRILOGA 1C

NASLOVNA STRAN NAČRTA

PODATKI O GRADNJI

naziv gradnje

Ureditev ŠP NK Litija- 2 faza

kratek opis gradnje

Objekt bo v pritličju zunanjih gabaritov(67,2 x 6,52 m) ter višine JP(slemenena) 8,83 m.

VRSTE GRADNJE



NOVOGRADNJA - NOVOZGRAJEN OBJEKT

označiti vse ustrezne vrste gradnje



NOVOGRADNJA - PRIZIDAVA



REKONSTRUKCIJA



SPREMEMBA NAMEMBOSTI



ODSTRANITEV CELOTNEGA OBJEKTA



LEGALIZACIJA



MANJŠA REKONSTRUKCIJA

PODATKI O PROJEKTNi DOKUMENTACIJI

vrsta dokumentacije

PZI (projektna dokumentacija za gradnjo)

številka projekta

02/2026

PODATKI O NAČRTU

strokovno področje načrta

GRADBENE KONSTRUKCIJE

naziv načrta

2-NAČRT S PODROČJA GRADBENIŠTVA

številka načrta

GK-13/26

datum izdelave

JUNIJ 2026

datum spremembe

PODATKI O PROJEKTANTU NAČRTA

projektant načrta (naziv družbe)

ISOMEDIA, zavod sodobnih komunikacij

naslov

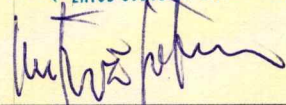
Zgornji prhovec 12a, IZLAKE

odgovorna oseba projektanta načrta

SOTENŠEK MATJAŽ inž. Gradb., G-1468

podpis odgovorne osebe

projektanta načrta

ISOMEDIA
ZAVOD SODOBNIH KOMUNIKACIJ

PODATKI O IZDELOVALCU NAČRTA

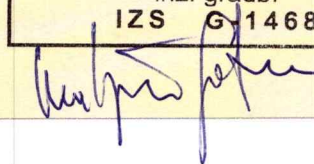
ime in priimek pooblaščenega arhitekta, pooblaščenega inženirja

MATJAŽ SOTENŠEK inž.gradb.

identifikacijska številka

G-1468

podpis pooblaščenega arhitekta, pooblaščenega inženirja

MATJAŽ SOTENŠEK
inž. gradb.
IZS G-1468

PRILOGA 2C

**IZJAVA PROJEKTANTA NAČRTA
IN POOBLAŠČENEGA STOKOVNJAKA,
KI JE IZDELAL NAČRT V PZI IN PID****PROJEKTANT NAČRTA**

projektant načrta (naziv družbe)	ISOMEDIA, zavod sodobnih komunikacij
naslov	Zgornji Prhovec 12a, 1411 Izlake
odgovorna oseba projektanta načrta	Matjaž Sotenšek, inž. Gradb.

IN POOBLAŠČENI STROKOVNJAK, KI JE IZDELAL NAČRT

pooblaščen strokovnjak	Matjaž Sotenšek, inž. Gradb.
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IZJAVLJAVA:**da načrt**

vrsta dokumentacije	PZI(projektna dokumentacija za izvedbo gradnje)
strokovno področje načrta	2.Načrt s področja gradbeništva
naziv načrta	2.1.Načrt gradbenih konstrukcij
številka načrta	G-13/26
datum izdelave	avg.26

upoštevam relevantne predpise in druge normativne dokumente ter da so upoštewane ustrezne bistvene in druge zahteve.

pooblaščen strokovnjak	Matjaž Sotenšek, inž. Gradb.
identifikacijska številka	G-1468
podpis pooblaščenega strokovnjaka	



odgovorna oseba projektanta načrta	Matjaž Sotenšek, inž. Gradb.
podpis odgovorne osebe projektanta načrta	



TEHNIČNO POROČILO

K statičnem računu za ureditev športnega parka NK Litija (IZVEDBA TRIBUN)-2faza

SPLOŠNO

Investitor Občina Litija, Jerebova ulica 14, 1270 Litija predvideva izgradnjo betonskih tribun in jekleno nadstrešnico nad delom betonskih tribun. Betonske tribune bodo v tlorskih izmerah 67,2x6,52 m in jeklena nadstrešnica v sredinskem delu dimenzij 34,8 x 6,52 m. Objekt bo pritličen. Višina nosilnega jeklenega stebra (okvirja) bo 8,83 m. Za potrebe investitorja se preveri ustreznost predpostavljene arhitekturne zasnove.

NOSILNA KONSTRUKCIJA

Sredinski del tribun bo pokrit z jekleno kritino iz TPO 900. Kritina se bo pritrdjevala z vijaki na vzdolžne strešne legice iz HEA 120 mm na medosni razdalji 1,0 m. Prečno na legice bodo jekleni okvirji na medosni razdalji 4,80 m.

Sestava jeklenih okvirjev:

-Jekleni konzolni stebri iz HEB 340 mm

-Jeklena zatega iz HEB 120 mm

-Jeklen nosilec iz HE 220 mm

Opis jeklenih nosilcev: Jeklena zatega iz HE 120B sidrana v jeklen konzolni steber iz HEB 340 mm na eni strani in na drugi strani na jeklen nosilec iz HEB 220 mm. Jeklen nosilec iz HEB 220 mm se bo sidral na jeklen steber iz HEB 340 mm z jekleno ploščo debeline 15 mm, na drugi strani ga bo nosila jeklena zatega iz HEB 120 mm. V zavetrovanih poljih strehe se kot pomožni profil uporabijo še jeklene zatege iz gladkega železa iz fi 14 mm.

OBTEŽBA

Stalno obtežbo na streho se upošteva kot 0,29 Kn/m². Veter se upošteva (projektna hitrost vetra 20m/s, II.kategorija terena). Sneg se upošteva nadmorska višina 300 m, 2,02 Kn/m² pri izračunu sil na jeklene legice. Lastna teža elementov armiranobetonske plošče in jeklenih elementov je avtomatsko upoštevana v programu. Stalna obtežba na armiranobetonsko ploščo tribun znaša na ravnem delu znaša 0,48 Kn/m² - do 6,13 Kn/m² na poševnem delu. Koristna obtežba na ravnem delu znaša 4,00 Kn/m² - do 4,38 Kn/m² na poševnem delu. Potresna obtežba na dotično geometrijo konstrukcijo vpliva (POZ 8). Na jekleno konstrukcijo je potresni vpliv zanemarljiv. Objekt se izvede v skladu z EC8.

TEMELJI

Temeljenje jeklenih konzolnih stebrov iz HEB 340 mm je na AB vertikalnih točkovnih temeljih dimenzij 1,2x1,2x2,0 m, ki so pravokotno povezani z horizontalnimi točkovnimi temelji dimenzij 1,2x1,2x300 m. Kvaliteta betona je C25/30 XC2. Točkovni AB temelji bodo na

STATIČNI RAČUN

imp 20260318 - 0002.polt

POZ 200-TRIBUNE NK LITIJA -

1.0. Ostrešje

1.1. Obtežba

a.) stalna

Jeklena kritina(TPO 900)	=	0,07 kN/m ²
Protikondenčni obrizg	=	0,10 kN/m ²
zaščita obrizga+instalacije	=	0,12 kN/m ²
g	=	0,29 kN/m ²

b.) sneg

cona	=	A2 ▼	
n.v.	=	300 m	
sk	=	1,51 kN/m ²	
α	=	10 °	
μ	=	$0,80 * (60 - \alpha) / 30$	= 1,33
s	=	sk * μ	= 2,02 kN/m ²

c.) veter

$V_{b,o}$	=	20 m/s	
C_e	=	1,7	
C_{dir}	=	1	
C_{season}	=	1	
V_b	=	$V_{b,o} * C_{dir} * C_{season}$	= 20 m/s
q_b	=	$0,5 * 1,25 * V_b^2$	= 0,25 kN/m ²
w	=	$q_b * C_e$	= 0,43 kN/m ²

TRIMOVAL plošče iz profilirane pločevine z obrizgom

POZ ① JEKLENA KRITINA FE 0,6 MM, S235 TPO 900

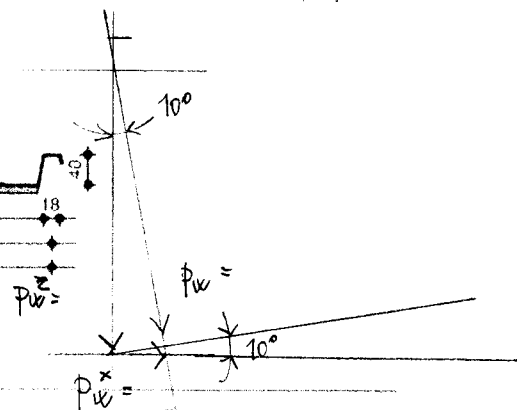
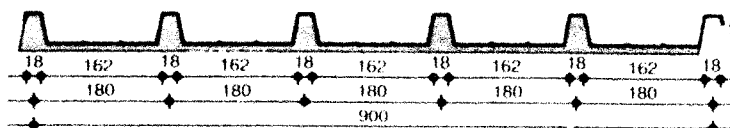
TPO je lahka gradbena plošča iz obarvane, profilirane pločevine (jeklene pocinkane ali aluminijaste), z 8 - 10 mm debelim poliuretanskim obrizgom.

Obrizg preprečuje nastajanje kondenza in podaljšuje življenjsko dobo kritini in ostrešju. Plošča je lahko tudi brez obrizga, to je TP 900.

OBTEŽBA : A/STALNA

$$(G + Q_s + Q_w)$$

$$\Delta = 10^\circ (Q_{uy}) = \sin 80^\circ \times 0,43 = 0,984 \times 0,43 = 0,423 \frac{\text{KN}}{\text{m}}$$



Tehnični podatki

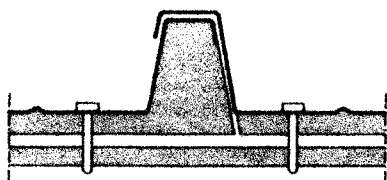
s	debelina pločevine	kg/m ²	Fe 0,6	Al 0,7
g	teža	kg/m	6,22	2,70
f	površina preseka cm ² /m		5,60	2,43
Wx	odpornostni moment cm ³ /m		7,62	9,52
Jx	vztrajnostni moment cm ⁴ /m		5,09	6,37
			15,49	19,37

$$G + Q_s + Q_w = 0,29 + 2,02 + 0,423 = 2,73 \text{ kN/m}^2 < 2,9$$

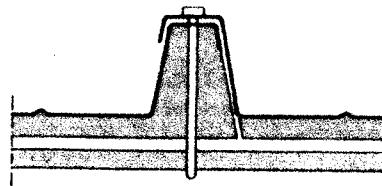
Dopustne obremenitve

debelina plošče (mm)		obloge	razmik med podporama	0,75	1,00	1,25	1,50	1,75
				obremenitev KN/m ²				
TPO	900	Fe 0,6	L (m)	3,60	2,90	2,60	2,30	2,20
TPO	900	Al 0,7	L (m)	2,00	1,70	1,60	1,40	1,30

Način pritrditve



Pritrditev TPO za fasadno oblogo



Pritrditev TPO za strešno kritino

POZ 2-JEKLEN NOSILEC IZ HE 120 A; S235

Shema konstrukcije

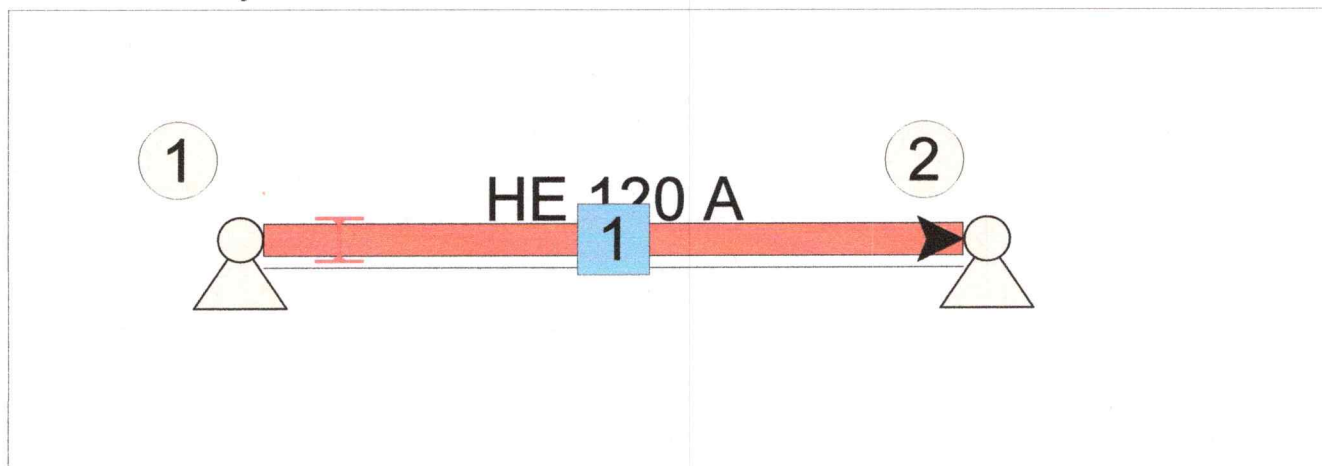


Tabela vozlišč

Index	X koor. [cm]	Z koor. [cm]	Podpore
1:	0.00000	0.00000	Ux Uz
2:	480.00	0.00000	Ux Uz

Tabela elementov

Index	Tip	Voz. 1	Voz. 2	Prečni prerez	Material	Dolžina [cm]	Spr. 1	Spr. 2
1:	nelin.	1:	2:	1: HE 120 A	1: S 235	480.00	-	-

Tabela materialov

Index	Elastični modul [kN/cm ²]	Koef. termičnega raztezka [1/d°C]
1: S 235	21000.	1.2000E-05

Tabela prečnih prerezov

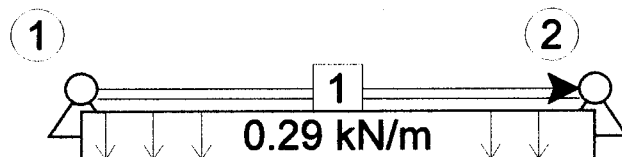
Index	A [cm ²]	I _y [cm ⁴]	Višina [cm]
1: HE 120 A	25.340	606.20	11.400

Obtežni vplivi in obtežni primeri

Obtežni primer	Mejno stanje	G	Qs	Qw
G+Q	nosilnosti	1.35	1.50	0.90
G+Q/ser	uporabnosti	1.00	1.00	0.60

A1 : G

G + Lastna teža je avtomatično upoštevana.

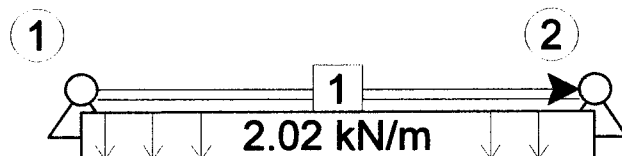


A1: G: Enakomerna zvezna obtežba elementov

Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Smer	Začetek	Konec	Lega
1:	0.00000	0.00000	0.29000	0.29000	0.00000	0.00000	Lokalni	0.00	1.00	Lokalni

A2 : Qs

Qs

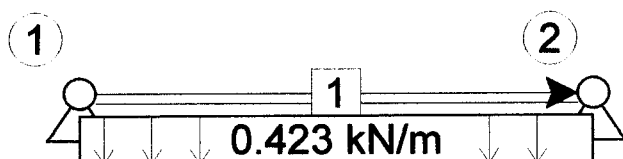


A2: Qs: Enakomerna zvezna obtežba elementov

Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Smer	Začetek	Konec	Lega	Ir
1:	0.00000	0.00000	2.0200	2.0200	0.00000	0.00000	Lokalni	0.00	1.00	Lokalni	C

A3 : Qw

Qw

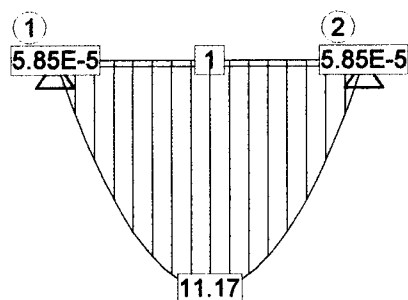


A3: Qw: Enakomerna zvezna obtežba elementov

Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Smer	Začetek	Konec	Lega
1:	0.00000	0.00000	0.42300	0.42300	0.00000	0.00000	Lokalni	0.00	1.00	Lokalni

LC1: G+Q: Upogibni moment My

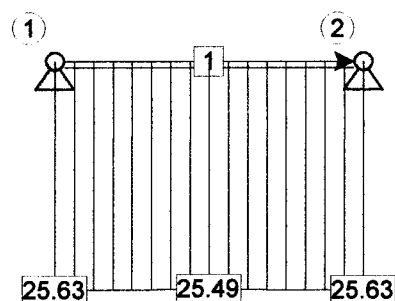
1.35 G + 1.50 Qs + 0.90 Qw



Enote: kNm

LC1: G+Q: Osnova sila Fx

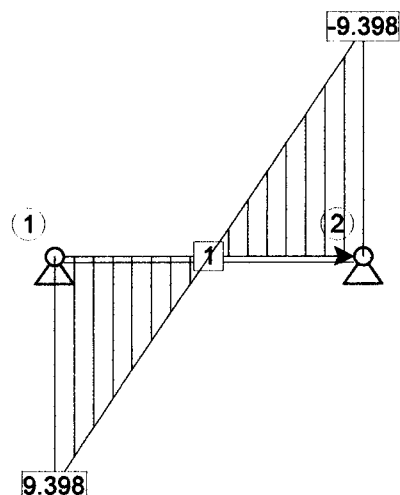
$$1.35 G + 1.50 Q_s + 0.90 Q_w$$



Enote: kN

LC1: G+Q: Prečna sila Fz

$$1.35 G + 1.50 Q_s + 0.90 Q_w$$



Enote: kN

LC1: G+Q: Vozlišni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
1:	-	-	-0.80600
2:	-	-	0.80600

LC1: G+Q: Reakcije

Voz.	Fx [kN]	Fz [kN]	My [kNm]
1:	-25.491	-9.7575	-
2:	25.491	-9.7575	-

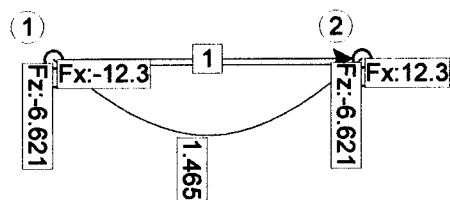
LC1: G+Q: Element 1:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	25.626	9.3980	5.8504E-05
0.05	25.610	8.4276	2.1390
0.10	25.593	7.4669	4.0462
0.15	25.574	6.5149	5.7239
0.20	25.555	5.5704	7.1741
0.25	25.537	4.6323	8.3984
0.30	25.522	3.6995	9.3982
0.35	25.509	2.7709	10.175
0.40	25.499	1.8455	10.729
0.45	25.493	0.92224	11.061
0.50	25.491	0.00000	11.171

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	25.493	-0.92224	11.061
0.60	25.499	-1.8455	10.729
0.65	25.509	-2.7709	10.175
0.70	25.522	-3.6995	9.3982
0.75	25.537	-4.6323	8.3984
0.80	25.555	-5.5704	7.1741
0.85	25.574	-6.5149	5.7239
0.90	25.593	-7.4669	4.0462
0.95	25.610	-8.4276	2.1390
1.00	25.626	-9.3980	5.8504E-05

LC2: G+Q/ser: Pomiki in reakcije

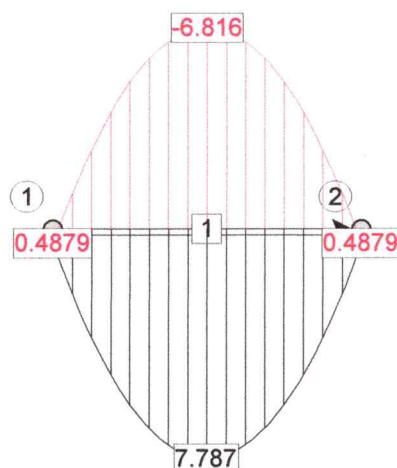
1.00 G + 1.00 Qs + 0.60 Qw



Enote: cm, kN, kNm

LC2: G+Q/ser: Napetost

1.00 G + 1.00 Qs + 0.60 Qw



Enote: kN/cm²

LC2: G+Q/ser: Vozlišni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
1:	-	-	-0.55973
2:	-	-	0.55973

LC2: G+Q/ser: Reakcije

Voz.	Fx [kN]	Fz [kN]	My [kNm]
1:	-12.301	-6.6215	-
2:	12.301	-6.6215	-

LC2: G+Q/ser: Element 1:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	12.365	6.5010	1.4603E-05
0.05	12.358	5.8406	1.4810
0.10	12.349	5.1835	2.8038
0.15	12.340	4.5293	3.9693
0.20	12.331	3.8776	4.9781
0.25	12.323	3.2281	5.8308
0.30	12.315	2.5803	6.5278
0.35	12.309	1.9340	7.0695
0.40	12.305	1.2887	7.4562
0.45	12.302	0.64420	7.6882
0.50	12.301	0.00000	7.7655

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	12.302	-0.64420	7.6882
0.60	12.305	-1.2887	7.4562
0.65	12.309	-1.9340	7.0695
0.70	12.315	-2.5803	6.5278
0.75	12.323	-3.2281	5.8308
0.80	12.331	-3.8776	4.9781
0.85	12.340	-4.5293	3.9693
0.90	12.349	-5.1835	2.8038
0.95	12.358	-5.8406	1.4810
1.00	12.365	-6.5010	1.4603E-05

Rezultati za element 1 (Preverjeno po EC3):

Prečni prerez [cm]

[HE 120 A], $h = 11.40$, $b = 12.00$, $tw = 0.50$, $tf = 0.80$, $r = 1.20$

Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Jeklo: S 235

Podpore in uklonska polja okoli osi y

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

Podpore in uklonska polja okoli osi z

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

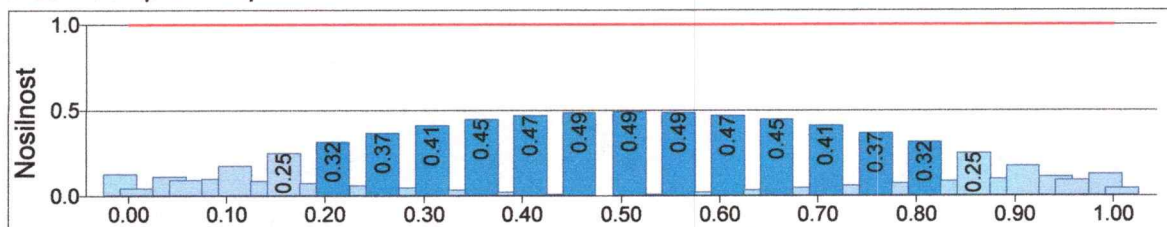
Podpore in uklonska polja za bočno zvrnitev

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

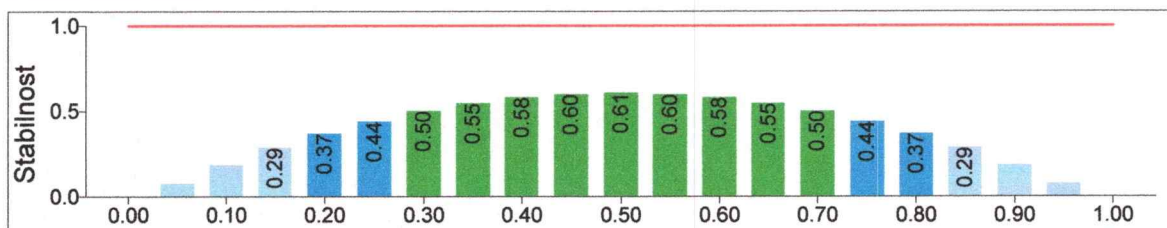
Relativna lega obtežbe za BZ je -1.00 (neugodno).

Mejno stanje nosilnosti

Nosilnosti prečnih prerezov



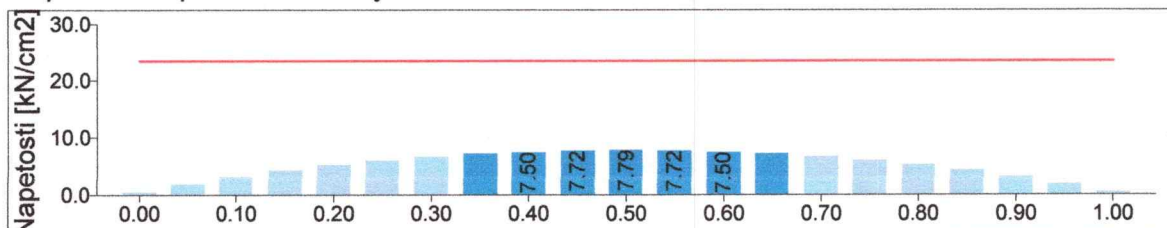
Stabilnost elementa



Element je bil razvrščen v razred 1. Vse kontrole stabilnosti so bile uspešne. Merodajna kontrola je kontrola bočne zvrnitve $[0.606 \leq 1.0]$ za obtežni primer '[U] G+Q'.

Mejno stanje uporabnosti

Napetosti v uporabnem stanju

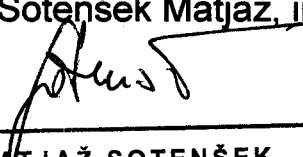


POZ 3-JEKLEN OKVIR IZ HE 340 B-STEBER, NOSILEC HE 220 B IN ZATEGA HE 120 B; S235

Projekt: Ureditev športnega parka NK Litija-2faza

Investitor: Občina Litija, Jerebova ulica 14, 1270 Litija

Avtor: Sotenšek Matjaž, inž.gr.


MATJAŽ SOTENŠEK
inž. gradb.
IZS G-1468

Komentar:

Opomba; Vsa dela lahko izvajajo le pooblašteni izvajalci. Vsako spremembo v projektu je potrebno evidentirati v gradbeni dnevnik in potrditi z strani pooblaščne organizacije (Nadzor) !

POZ 3-JEKLEN OKVIR IZ HE 340 B-STEBER, NOSILEC HE 220 B IN ZATEGA HE 120 B; S235

Shema konstrukcije

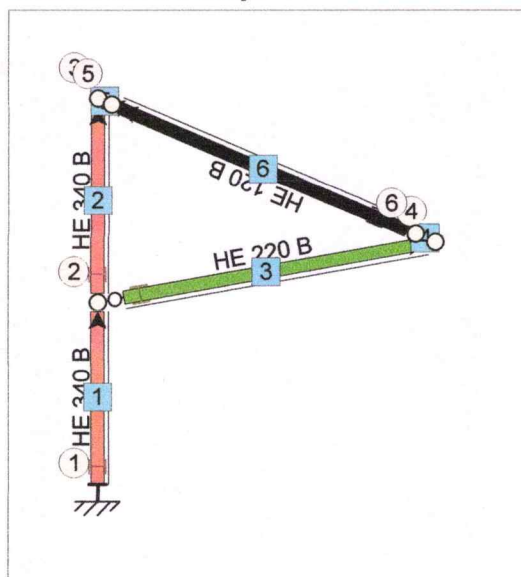


Tabela vozlišč

Index	X koor. [cm]	Z koor. [cm]	Podpore
1:	0.00000	0.00000	Ux Uz Ry
2:	0.00000	-320.00	-
3:	0.00000	-660.00	-

Index	X koor. [cm]	Z koor. [cm]	Podpore
4:	560.00	-420.00	-
5:	22.400	-650.40	-
6:	527.74	-433.82	-

Tabela elementov

Index	Tip	Voz. 1	Voz. 2	Prečni prerez	Material	Dolžina [cm]	Spr. 1	Spr. 2
1:	nelin.	1:	2:	1: HE 340 B	1: S 355	320.00	-	-
2:	nelin.	2:	3:	1: HE 340 B	1: S 355	340.00	-	-
3:	nelin.	2:	4:	2: HE 220 B	1: S 355	568.86	Ry	-
4:	nelin.	4:	6:	1: HE 340 B	1: S 355	35.093	-	-
5:	nelin.	5:	3:	1: HE 340 B	1: S 355	24.370	-	-
6:	nelin.	6:	5:	3: HE 120 B	1: S 355	549.80	-	-

Tabela materialov

Index	Elastični modul [kN/cm ²]	Koef. termičnega raztezka [1/d°C]
1: S 355	21000.	1.2000E-05

Tabela prečnih prerezov

Index	A [cm ²]	I _y [cm ⁴]	Višina [cm]
1: HE 340 B	170.90	36660.	34.000
2: HE 220 B	91.040	8091.0	22.000

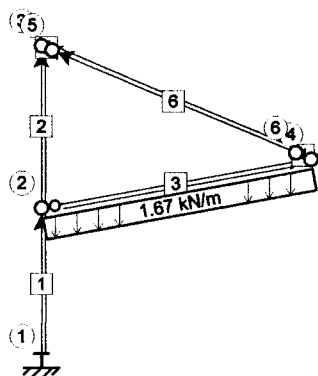
Index	A [cm ²]	I _y [cm ⁴]	Višina [cm]
3: HE 120 B	34.010	864.40	12.000

Obtežni vplivi in obtežni primeri

Obtežni primer	Mejno stanje	G	Qs	Qw
G+Q	nosilnosti	1.35	1.50	0.90
G+Q/ser	uporabnosti	1.00	1.00	0.60

A1 : G

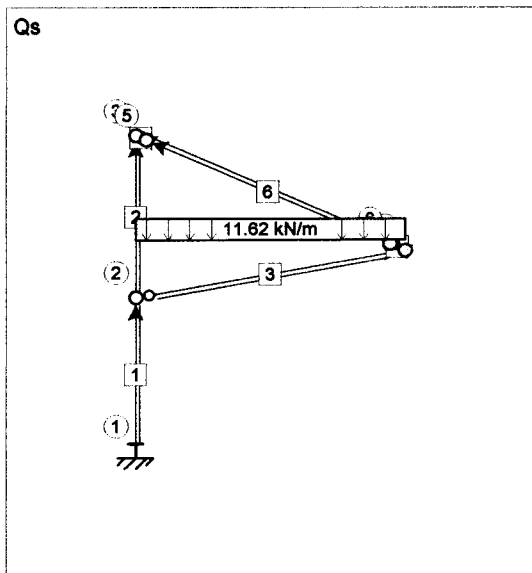
G + Lastna teža je avtomatično upoštevana.



A1: G: Enakomerna zvezna obtežba elementov

Element	F _{x s} [kN/m]	F _{x e} [kN/m]	F _{z s} [kN/m]	F _{z e} [kN/m]	M _{ys} [kNm/m]	M _{ye} [kNm/m]	Smer	Začetek	Konec	Lega	l
3:	0.00000	0.00000	1.6700	1.6700	0.00000	0.00000	Globalni	0.00	1.00	Lokalni	

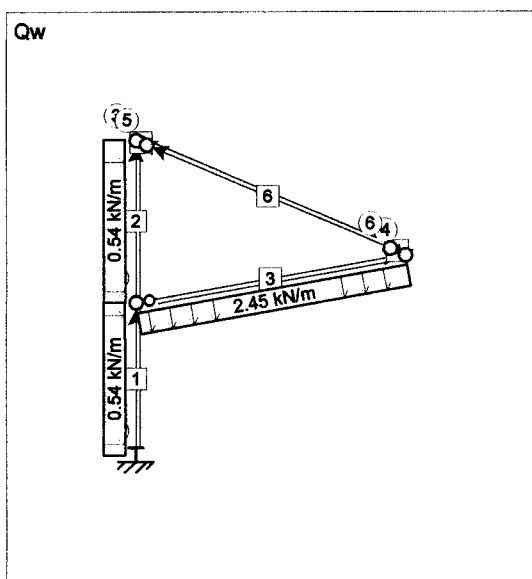
A2 : Qs



A2: Qs: Enakomerna zvezna obtežba elementov

Element	$F_x s$ [kN/m]	$F_x e$ [kN/m]	$F_z s$ [kN/m]	$F_z e$ [kN/m]	$M_y s$ [kNm/m]	$M_y e$ [kNm/m]	Smer	Začetek	Konec	Lega
3:	0.00000	0.00000	11.620	11.620	0.00000	0.00000	Globalni	0.00	1.00	Globalni

A3 : Qw

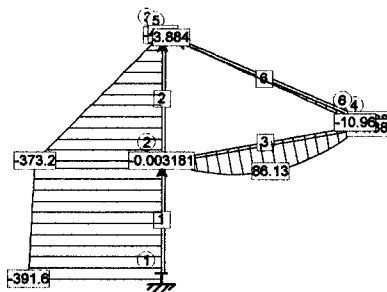


A3: Qw: Enakomerna zvezna obtežba elementov

Element	$F_x s$ [kN/m]	$F_x e$ [kN/m]	$F_z s$ [kN/m]	$F_z e$ [kN/m]	$M_y s$ [kNm/m]	$M_y e$ [kNm/m]	Smer	Začetek	Konec	Lega
1:	0.00000	0.00000	0.54000	0.54000	0.00000	0.00000	Lokalni	0.00	1.00	Lokalni
2:	0.00000	0.00000	0.54000	0.54000	0.00000	0.00000	Lokalni	0.00	1.00	Lokalni
3:	0.00000	0.00000	2.4500	2.4500	0.00000	0.00000	Lokalni	0.00	1.00	Lokalni

LC1: G+Q: Upogibni moment M_y

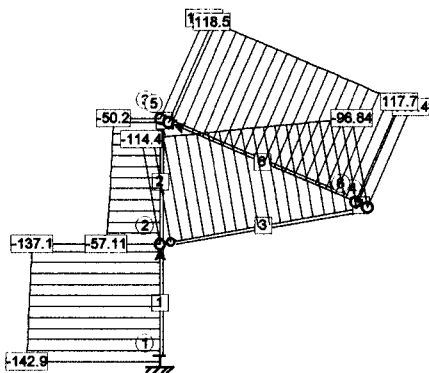
1.35 G + 1.50 Qs + 0.90 Qw



Enote: kNm

LC1: G+Q: Osna sila F_x

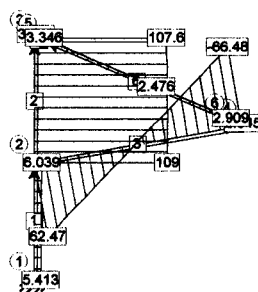
1.35 G + 1.50 Qs + 0.90 Qw



Enote: kN

LC1: G+Q: Prečna sila F_z

1.35 G + 1.50 Qs + 0.90 Qw



Enote: kN

LC1: G+Q: Vozliščni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
2:	2.5648	0.026135	-0.91118
3:	9.8484	0.11028	-1.3885
4:	4.5708	12.281	-0.73873

Voz.	ux [cm]	uz [cm]	fiy [deg]
5:	9.6099	0.65072	-1.3892
6:	4.7511	11.865	-0.74172

LC1: G+Q: Reakcije

Voz.	Fx [kN]	Fz [kN]	My [kNm]
1:	-5.4126	-142.89	391.63

LC1: G+Q: Element 1:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-142.89	5.4126	-391.63
0.05	-142.60	5.4508	-390.76
0.10	-142.31	5.4882	-389.89
0.15	-142.03	5.5250	-389.01
0.20	-141.74	5.5610	-388.12
0.25	-141.45	5.5963	-387.23
0.30	-141.16	5.6308	-386.33
0.35	-140.87	5.6647	-385.43
0.40	-140.58	5.6978	-384.52
0.45	-140.30	5.7302	-383.60
0.50	-140.01	5.7618	-382.68

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	-139.72	5.7928	-381.76
0.60	-139.43	5.8230	-380.83
0.65	-139.14	5.8525	-379.90
0.70	-138.85	5.8813	-378.96
0.75	-138.57	5.9094	-378.01
0.80	-138.28	5.9367	-377.07
0.85	-137.99	5.9633	-376.11
0.90	-137.70	5.9892	-375.16
0.95	-137.42	6.0144	-374.20
1.00	-137.13	6.0388	-373.23

LC1: G+Q: Element 2:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-57.108	108.96	-373.23
0.05	-56.720	108.92	-354.71
0.10	-56.336	108.87	-336.20
0.15	-55.957	108.83	-317.70
0.20	-55.583	108.78	-299.20
0.25	-55.213	108.72	-280.71
0.30	-54.847	108.67	-262.24
0.35	-54.486	108.61	-243.77
0.40	-54.130	108.55	-225.31
0.45	-53.778	108.49	-206.86
0.50	-53.430	108.42	-188.43

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	-53.087	108.35	-170.00
0.60	-52.749	108.28	-151.59
0.65	-52.415	108.21	-133.18
0.70	-52.085	108.13	-114.80
0.75	-51.760	108.06	-96.420
0.80	-51.439	107.98	-78.058
0.85	-51.123	107.89	-59.709
0.90	-50.811	107.81	-41.375
0.95	-50.503	107.72	-23.055
1.00	-50.200	107.63	-4.7498

LC1: G+Q: Element 3:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-114.43	62.465	-0.0031809
0.05	-113.62	56.092	16.860
0.10	-112.82	49.689	31.905
0.15	-112.03	43.261	45.124
0.20	-111.23	36.810	56.510
0.25	-110.43	30.341	66.059
0.30	-109.62	23.859	73.766
0.35	-108.80	17.366	79.628
0.40	-107.96	10.866	83.643
0.45	-107.10	4.3626	85.809
0.50	-106.23	-2.1402	86.126

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	-105.33	-8.6394	84.593
0.60	-104.42	-15.132	81.213
0.65	-103.50	-21.614	75.988
0.70	-102.56	-28.083	68.920
0.75	-101.60	-34.537	60.014
0.80	-100.65	-40.972	49.274
0.85	-99.684	-47.386	36.708
0.90	-98.725	-53.776	22.321
0.95	-97.775	-60.140	6.1224
1.00	-96.842	-66.476	-11.880

LC1: G+Q: Element 4:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	117.44	2.3450	-11.884
0.05	117.45	2.3732	-11.842
0.10	117.46	2.4014	-11.801
0.15	117.48	2.4296	-11.758
0.20	117.49	2.4577	-11.715
0.25	117.50	2.4859	-11.672
0.30	117.51	2.5141	-11.628
0.35	117.53	2.5423	-11.584
0.40	117.54	2.5705	-11.539
0.45	117.55	2.5986	-11.493
0.50	117.57	2.6268	-11.448

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	117.58	2.6550	-11.401
0.60	117.59	2.6832	-11.354
0.65	117.60	2.7114	-11.307
0.70	117.62	2.7396	-11.259
0.75	117.63	2.7678	-11.211
0.80	117.64	2.7960	-11.162
0.85	117.65	2.8242	-11.113
0.90	117.67	2.8524	-11.063
0.95	117.68	2.8806	-11.013
1.00	117.69	2.9088	-10.962

LC1: G+Q: Element 5:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	118.53	3.3462	3.8861
0.05	118.54	3.3660	3.9270
0.10	118.54	3.3857	3.9681
0.15	118.55	3.4055	4.0095
0.20	118.56	3.4252	4.0511
0.25	118.57	3.4450	4.0930
0.30	118.58	3.4648	4.1351
0.35	118.59	3.4845	4.1774
0.40	118.60	3.5043	4.2200
0.45	118.61	3.5241	4.2628

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.50	118.62	3.5438	4.3059
0.55	118.63	3.5636	4.3492
0.60	118.63	3.5834	4.3927
0.65	118.64	3.6031	4.4365
0.70	118.65	3.6229	4.4806
0.75	118.66	3.6427	4.5248
0.80	118.67	3.6625	4.5693
0.85	118.68	3.6822	4.6141
0.90	118.69	3.7020	4.6591
0.95	118.70	3.7218	4.7043

LC1: G+Q: Element 5:

Lega	Fx [kN]	Fz [kN]	My [kNm]
1.00	118.71	3.7416	4.7498

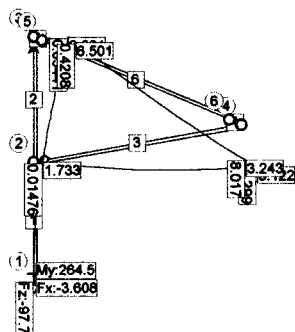
LC1: G+Q: Element 6:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	117.69	2.9088	-10.960
0.05	117.74	2.8092	-10.176
0.10	117.78	2.7233	-9.4166
0.15	117.82	2.6506	-8.6781
0.20	117.87	2.5908	-7.9575
0.25	117.91	2.5437	-7.2515
0.30	117.95	2.5089	-6.5569
0.35	117.99	2.4864	-5.8703
0.40	118.04	2.4761	-5.1884
0.45	118.08	2.4778	-4.5079
0.50	118.12	2.4917	-3.8254

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	118.16	2.5178	-3.1373
0.60	118.20	2.5562	-2.4403
0.65	118.25	2.6072	-1.7308
0.70	118.29	2.6710	-1.0054
0.75	118.33	2.7480	-0.26041
0.80	118.37	2.8385	0.50770
0.85	118.41	2.9430	1.3025
0.90	118.45	3.0620	2.1278
0.95	118.49	3.1962	2.9871
1.00	118.53	3.3462	3.8843

LC2: G+Q/ser: Pomiki in reakcije

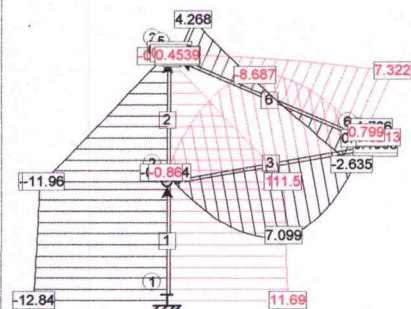
1.00 G + 1.00 Qs + 0.60 Qw



Enote: cm, kN, kNm

LC2: G+Q/ser: Napetost

1.00 G + 1.00 Qs + 0.60 Qw



Enote: kN/cm²

LC2: G+Q/ser: Vozlišni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
2:	1.7332	0.014757	-0.61609
3:	6.6613	0.054412	-0.93967
4:	3.1219	8.2988	-0.49967

Voz.	ux [cm]	uz [cm]	fiy [deg]
5:	6.5013	0.42080	-0.94022
6:	3.2433	8.0171	-0.50161

LC2: G+Q/ser: Reakcije

Voz.	Fx [kN]	Fz [kN]	My [kNm]
1:	-3.6084	-97.701	264.47

LC2: G+Q/ser: Element 1:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-97.701	3.6084	-264.47
0.05	-97.488	3.6101	-263.89
0.10	-97.276	3.6114	-263.31
0.15	-97.063	3.6124	-262.73
0.20	-96.851	3.6131	-262.16
0.25	-96.638	3.6133	-261.58
0.30	-96.426	3.6133	-261.00
0.35	-96.214	3.6129	-260.42
0.40	-96.001	3.6122	-259.84
0.45	-95.789	3.6111	-259.27
0.50	-95.577	3.6096	-258.69

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	-95.365	3.6079	-258.11
0.60	-95.152	3.6058	-257.53
0.65	-94.940	3.6033	-256.96
0.70	-94.728	3.6005	-256.38
0.75	-94.516	3.5974	-255.81
0.80	-94.304	3.5939	-255.23
0.85	-94.092	3.5901	-254.66
0.90	-93.880	3.5860	-254.08
0.95	-93.668	3.5815	-253.51
1.00	-93.456	3.5767	-252.94

LC2: G+Q/ser: Element 2:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-38.990	73.846	-252.94
0.05	-38.727	73.810	-240.38
0.10	-38.465	73.772	-227.84
0.15	-38.206	73.733	-215.30
0.20	-37.949	73.693	-202.77
0.25	-37.694	73.651	-190.25
0.30	-37.441	73.608	-177.73
0.35	-37.191	73.564	-165.22
0.40	-36.942	73.519	-152.72
0.45	-36.695	73.473	-140.22
0.50	-36.451	73.425	-127.74

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	-36.208	73.376	-115.26
0.60	-35.967	73.326	-102.79
0.65	-35.729	73.276	-90.330
0.70	-35.492	73.224	-77.877
0.75	-35.258	73.170	-65.434
0.80	-35.026	73.116	-53.000
0.85	-34.795	73.061	-40.575
0.90	-34.567	73.005	-28.159
0.95	-34.341	72.948	-15.753
1.00	-34.116	72.890	-3.3572

LC2: G+Q/ser: Element 3:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-78.312	42.089	-0.0015392
0.05	-77.717	37.783	11.359
0.10	-77.126	33.463	21.492
0.15	-76.538	29.131	30.393
0.20	-75.950	24.790	38.061
0.25	-75.358	20.440	44.493
0.30	-74.763	16.083	49.687
0.35	-74.161	11.722	53.641
0.40	-73.553	7.3577	56.354
0.45	-72.936	2.9920	57.826
0.50	-72.312	-1.3734	58.057

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	-71.679	-5.7372	57.046
0.60	-71.037	-10.098	54.794
0.65	-70.389	-14.454	51.302
0.70	-69.734	-18.804	46.573
0.75	-69.074	-23.147	40.606
0.80	-68.410	-27.481	33.406
0.85	-67.746	-31.806	24.974
0.90	-67.082	-36.120	15.314
0.95	-66.423	-40.422	4.4295
1.00	-65.771	-44.711	-7.6758

LC2: G+Q/ser: Element 4:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	79.520	1.2275	-7.6774
0.05	79.529	1.2485	-7.6557
0.10	79.538	1.2695	-7.6336
0.15	79.548	1.2905	-7.6111
0.20	79.557	1.3115	-7.5883
0.25	79.566	1.3325	-7.5651
0.30	79.575	1.3535	-7.5416
0.35	79.585	1.3746	-7.5176
0.40	79.594	1.3956	-7.4933
0.45	79.603	1.4166	-7.4686

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.50	79.613	1.4376	-7.4436
0.55	79.622	1.4586	-7.4182
0.60	79.631	1.4796	-7.3924
0.65	79.640	1.5006	-7.3663
0.70	79.650	1.5216	-7.3398
0.75	79.659	1.5426	-7.3129
0.80	79.668	1.5637	-7.2856
0.85	79.678	1.5847	-7.2580
0.90	79.687	1.6057	-7.2300
0.95	79.696	1.6267	-7.2016

LC2: G+Q/ser: Element 4:

Lega	Fx [kN]	Fz [kN]	My [kNm]
1.00	79.705	1.6477	-7.1729

LC2: G+Q/ser: Element 5:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	80.305	2.3515	2.7483
0.05	80.311	2.3662	2.7771
0.10	80.318	2.3809	2.8060
0.15	80.324	2.3956	2.8351
0.20	80.331	2.4102	2.8644
0.25	80.337	2.4249	2.8938
0.30	80.344	2.4396	2.9235
0.35	80.350	2.4542	2.9533
0.40	80.357	2.4689	2.9833
0.45	80.364	2.4836	3.0135
0.50	80.370	2.4983	3.0438

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	80.377	2.5129	3.0743
0.60	80.383	2.5276	3.1051
0.65	80.390	2.5423	3.1359
0.70	80.396	2.5570	3.1670
0.75	80.403	2.5716	3.1983
0.80	80.409	2.5863	3.2297
0.85	80.416	2.6010	3.2613
0.90	80.423	2.6157	3.2931
0.95	80.429	2.6303	3.3250
1.00	80.436	2.6450	3.3572

LC2: G+Q/ser: Element 6:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	79.705	1.6477	-7.1724
0.05	79.736	1.6297	-6.7225
0.10	79.767	1.6171	-6.2765
0.15	79.797	1.6098	-5.8330
0.20	79.828	1.6078	-5.3907
0.25	79.858	1.6111	-4.9482
0.30	79.889	1.6197	-4.5041
0.35	79.919	1.6337	-4.0569
0.40	79.949	1.6531	-3.6053
0.45	79.980	1.6779	-3.1476
0.50	80.010	1.7083	-2.6823

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	80.040	1.7443	-2.2079
0.60	80.070	1.7862	-1.7228
0.65	80.100	1.8340	-1.2253
0.70	80.130	1.8879	-0.71381
0.75	80.159	1.9481	-0.18655
0.80	80.189	2.0148	0.35820
0.85	80.218	2.0882	0.92218
0.90	80.247	2.1686	1.5072
0.95	80.276	2.2563	2.1151
1.00	80.305	2.3515	2.7478

Rezultati za element 1 (Preverjeno po EC3):

Prečni prerez [cm]

[HE 340 B], h = 34.00, b = 30.00, tw = 1.20, tf = 2.15, r = 2.70

Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Jeklo: S 355

Podpore in uklonska polja okoli osi y

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

Podpore in uklonska polja okoli osi z

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

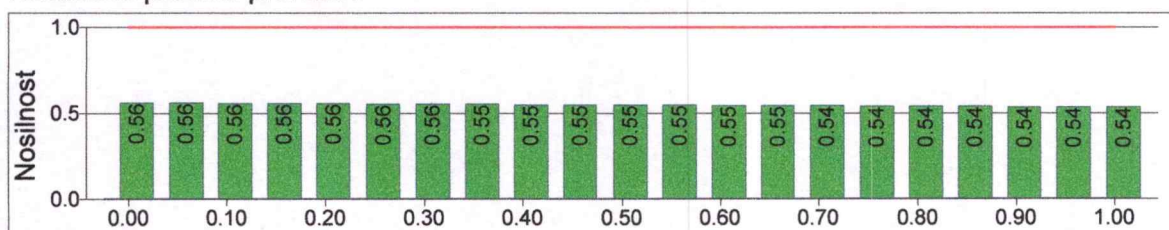
Podpore in uklonska polja za bočno zvrnitev

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

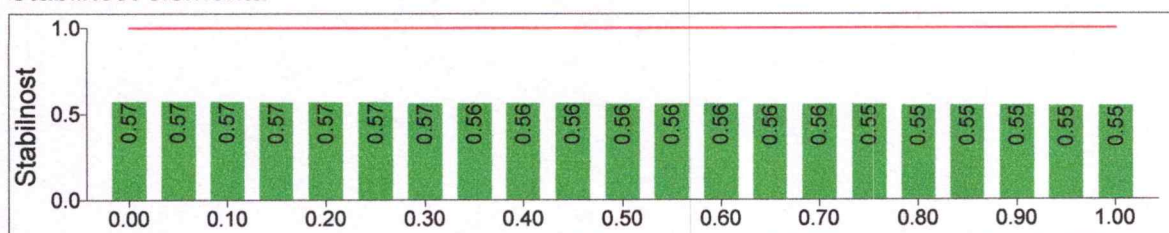
Relativna lega obtežbe za BZ je -1.00 (neugodno).

Mejno stanje nosilnosti

Nosilnosti prečnih prereзов



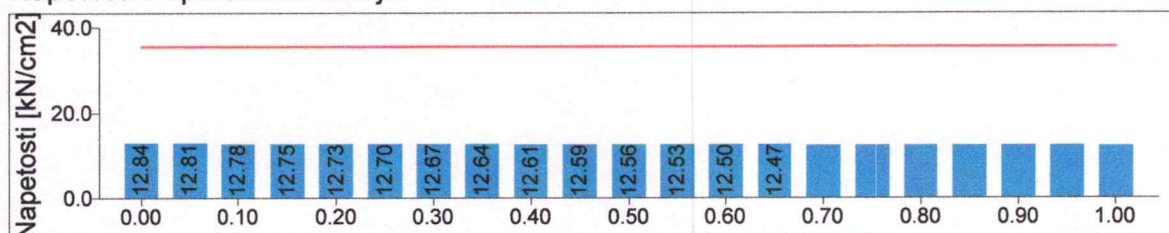
Stabilnost elementa



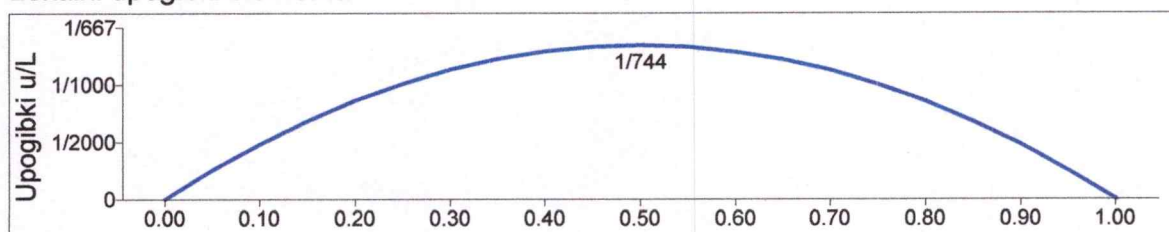
Element je bil razvrščen v razred 1. Vse kontrole stabilnosti so bile uspešne. Merodajna kontrola je kontrola stabilnosti na tlačno osno silo in bočno zvrnitev [$0.573 \leq 1.0$] za obtežni primer '[U] G+Q'.

Mejno stanje uporabnosti

Napetosti v uporabnem stanju



Lokalni upogibki elementa



Rezultati za element 2 (Preverjeno po EC3):

Prečni prerez [cm]

-21-

Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Jeklo: S 355

Podpore in uklonska polja okoli osi y

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

Podpore in uklonska polja okoli osi z

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

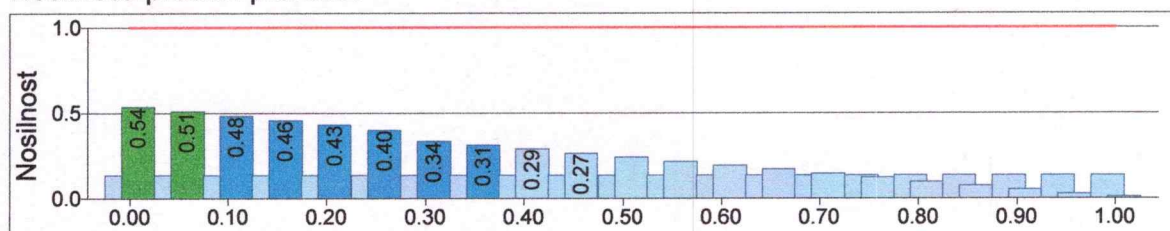
Podpore in uklonska polja za bočno zvrnitev

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

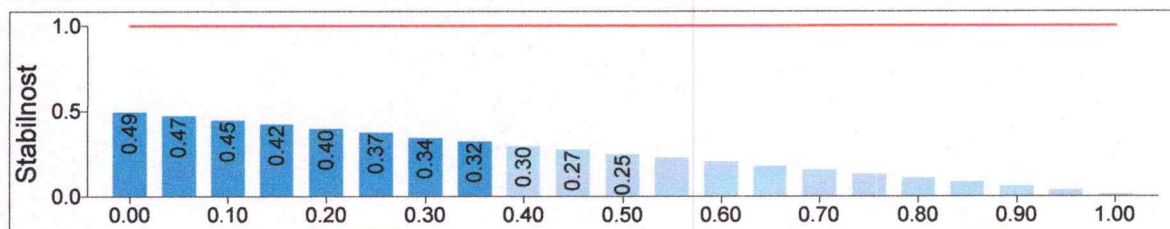
Relativna lega obtežbe za BZ je -1.00 (neugodno).

Mejno stanje nosilnosti

Nosilnosti prečnih prerezov



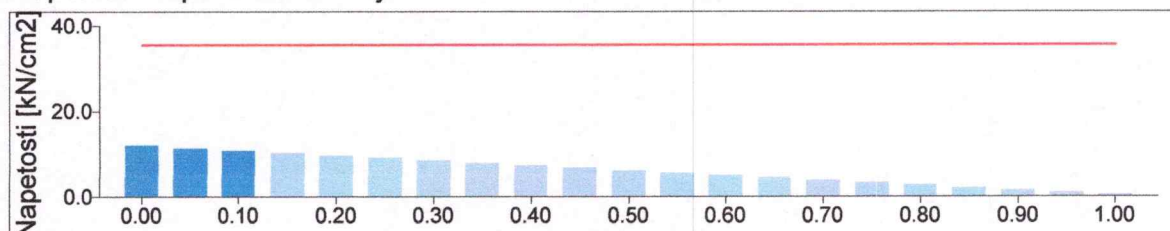
Stabilnost elementa



Element je bil razvrščen v razred 1. Vse kontrole stabilnosti so bile uspešne. Merodajna kontrola je kontrola stabilnosti na tlačno osno silo in upogibni moment $[0.493 \leq 1.0]$ za obtežni primer '[U] G+Q'.

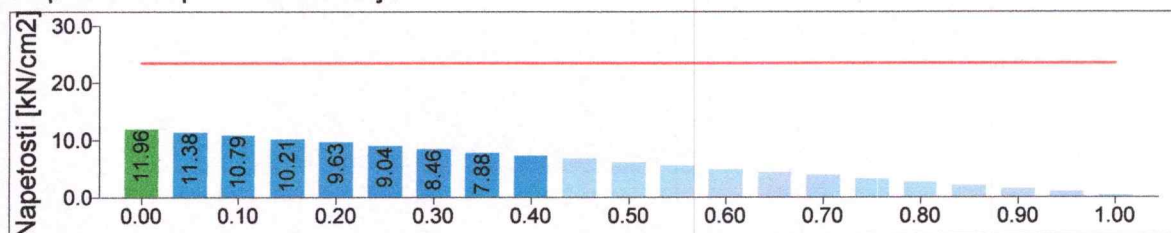
Mejno stanje uporabnosti

Napetosti v uporabnem stanju

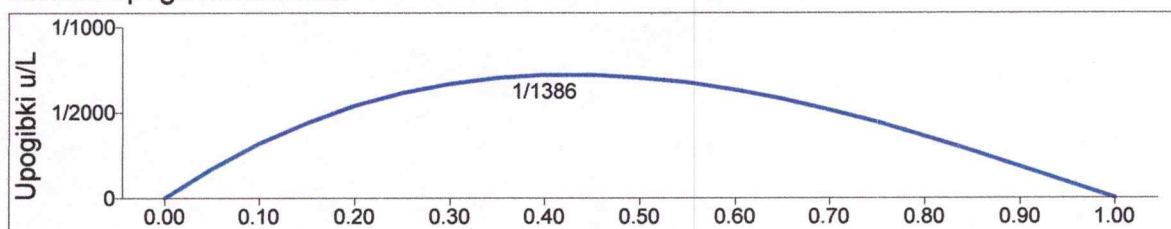


Mejno stanje uporabnosti

Napetosti v uporabnem stanju



Lokalni upogibki elementa



Rezultati za element 3 (Preverjeno po EC3):

Prečni prerez [cm]

[HE 220 B], $h = 22.00$, $b = 22.00$, $t_w = 0.95$, $t_f = 1.60$, $r = 1.80$

Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Jeklo: S 235

Podpore in uklonska polja okoli osi y

Polje	x začetek	x konec	ukloni koef.
1	0.000	1.000	1.000

Podpore in uklonska polja okoli osi z

Polje	x začetek	x konec	ukloni koef.
1	0.000	1.000	1.000

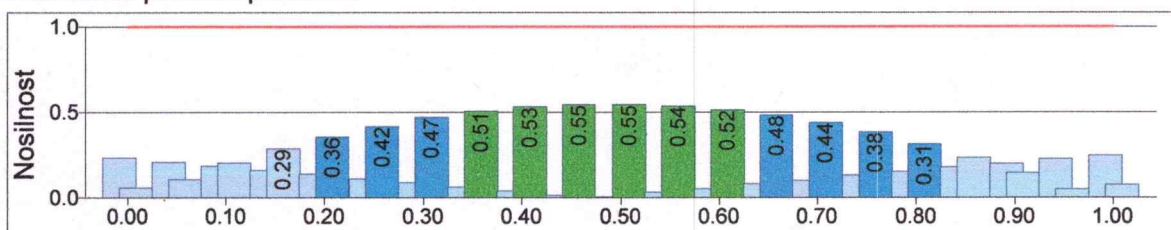
Podpore in uklonska polja za bočno zvrnitev

Polje	x začetek	x konec	ukloni koef.
1	0.000	1.000	1.000

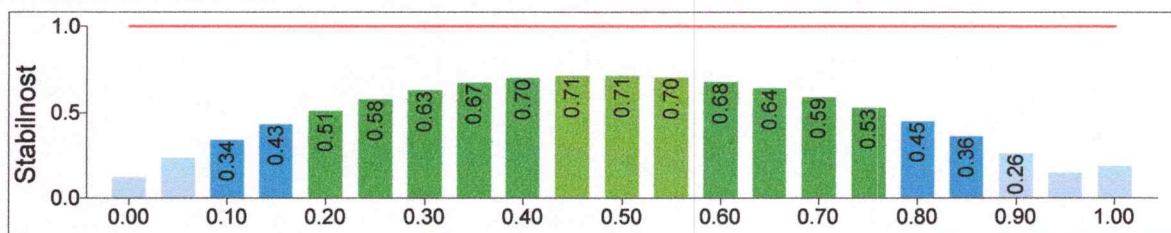
Relativna lega obtežbe za BZ je -1.00 (neugodno).

Mejno stanje nosilnosti

Nosilnosti prečnih prerezov



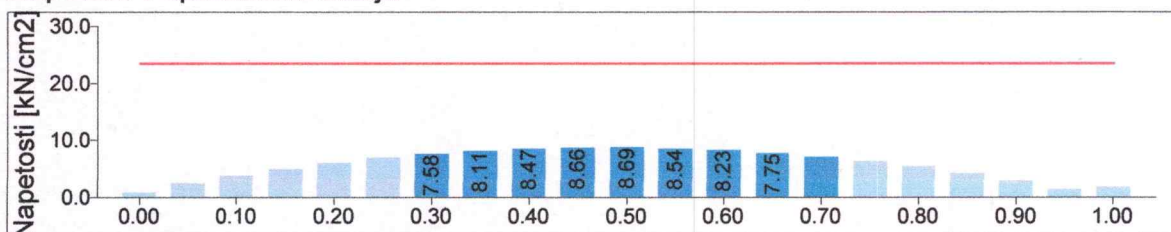
Stabilnost elementa



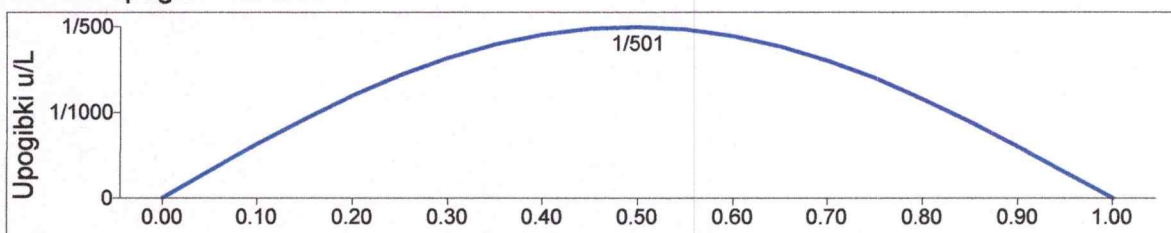
Element je bil razvrščen v razred 1. Vse kontrole stabilnosti so bile uspešne. Merodajna kontrola je kontrola stabilnosti na tlačno osno silo in bočno zvrnitev $[0.715 \leq 1.0]$ za obtežni primer '[U] G+Q'.

Mejno stanje uporabnosti

Napetosti v uporabnem stanju



Lokalni upogibki elementa



Rezultati za element 4 (Preverjeno po EC3):

Prečni prerez [cm]

[HE 340 B], $h = 34.00$, $b = 30.00$, $t_w = 1.20$, $t_f = 2.15$, $r = 2.70$

Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Jeklo: S 235

Podpore in uklonska polja okoli osi y

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

Podpore in uklonska polja okoli osi z

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

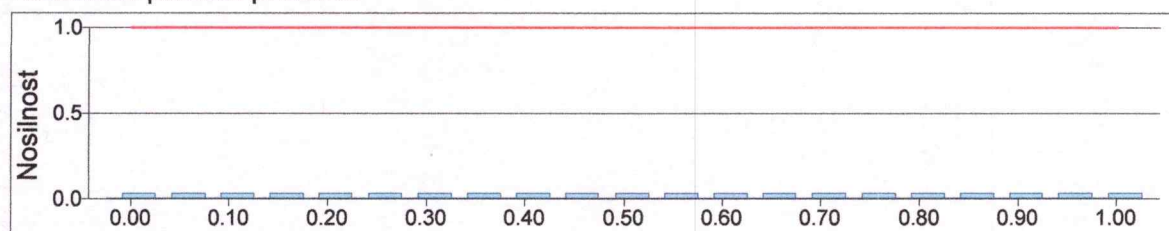
Podpore in uklonska polja za bočno zvrnitev

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

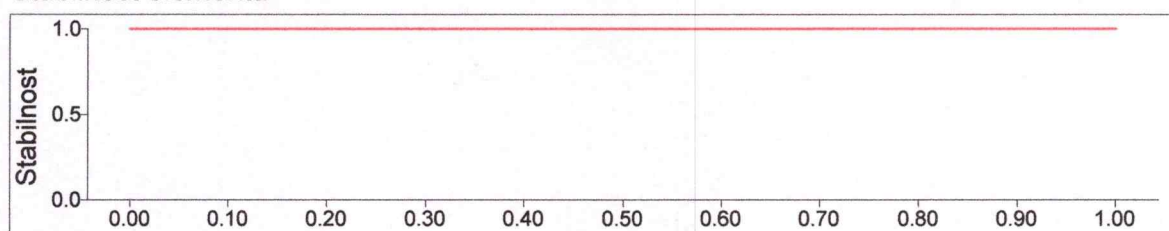
Relativna lega obtežbe za BZ je -1.00 (neugodno).

Mejno stanje nosilnosti

Nosilnosti prečnih prerezov



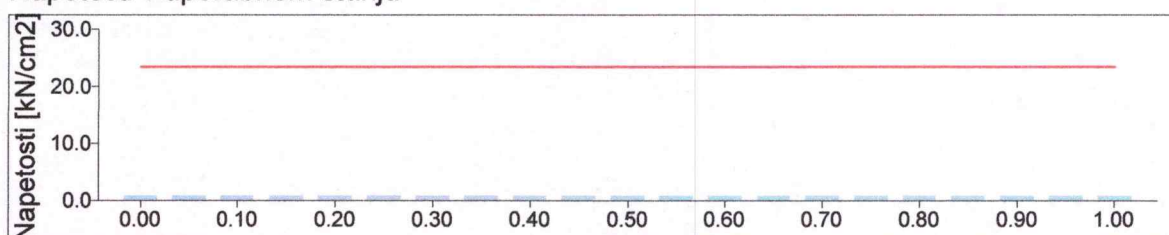
Stabilnost elementa



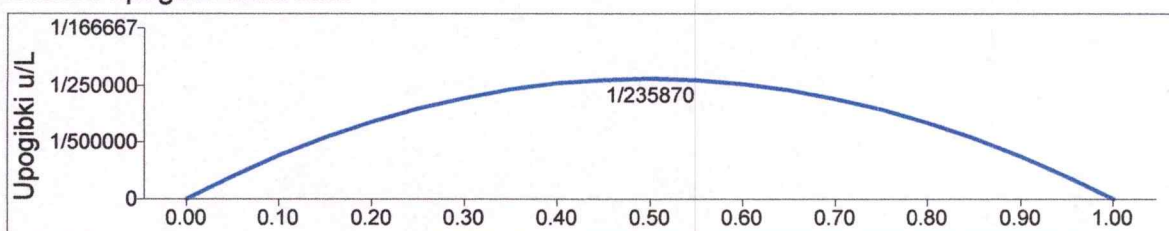
Element je bil razvrščen v razred 1. Vse kontrole stabilnosti so bile uspešne. Merodajna kontrola je kontrola bočne zvrnitve $[0.000 \leq 1.0]$ za obtežni primer '[U] G+Q'.

Mejno stanje uporabnosti

Napetosti v uporabnem stanju



Lokalni upogibki elementa



Rezultati za element 5 (Preverjeno po EC3):

Prečni prerez [cm]

[HE 340 B], $h = 34.00$, $b = 30.00$, $t_w = 1.20$, $t_f = 2.15$, $r = 2.70$

Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Jeklo: S 235

Podpore in uklonska polja okoli osi y

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

Podpore in uklonska polja okoli osi z

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

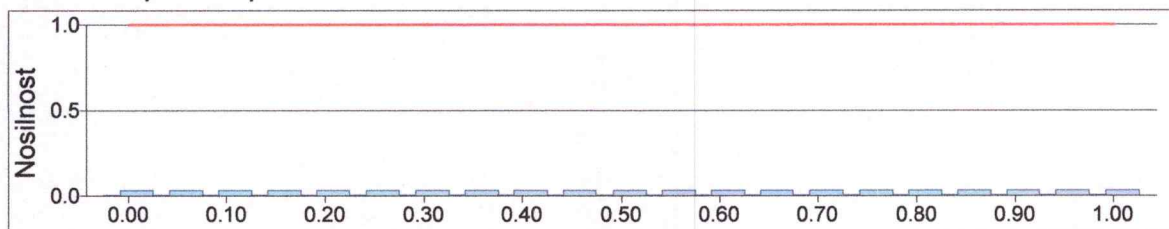
Podpore in uklonska polja za bočno zvrnitev

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

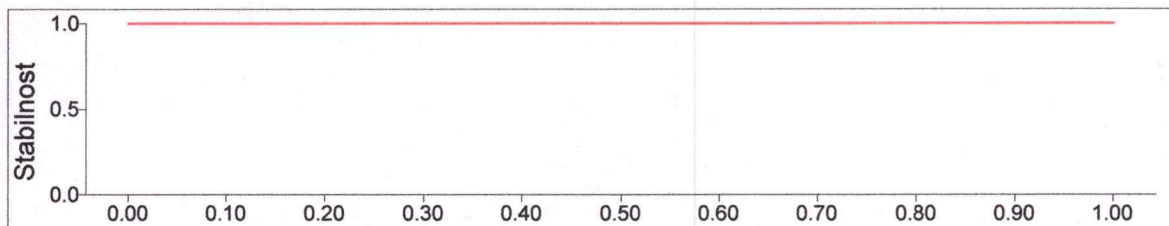
Relativna lega obtežbe za BZ je -1.00 (neugodno).

Mejno stanje nosilnosti

Nosilnosti prečnih prerezov



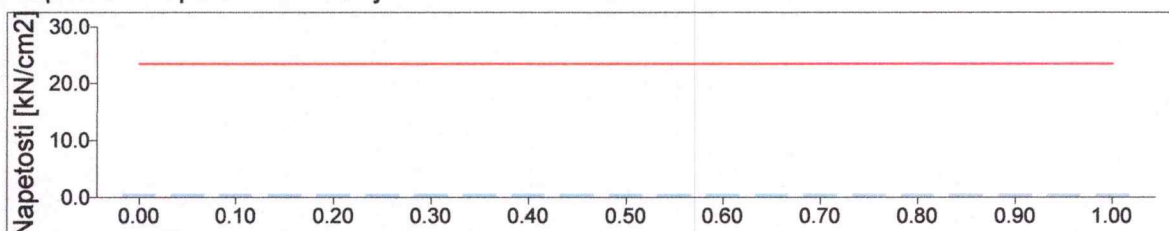
Stabilnost elementa



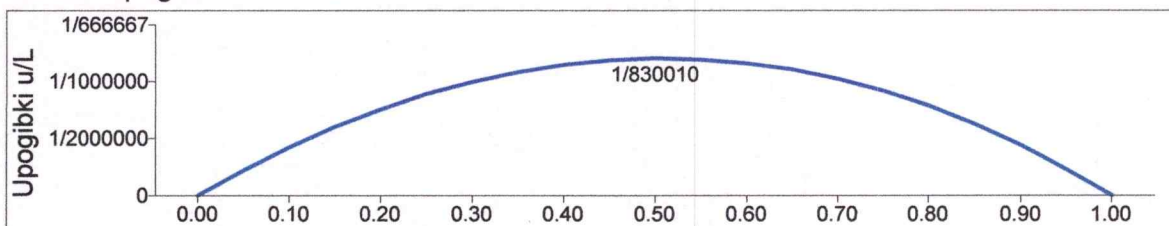
Stabilnost elementa ni bila kontrolirana.

Mejno stanje uporabnosti

Napetosti v uporabnem stanju



Lokalni upogibki elementa



Rezultati za element 6 (Preverjeno po EC3):

Prečni prerez [cm]

[HE 120 B], $h = 12.00$, $b = 12.00$, $t_w = 0.65$, $t_f = 1.10$, $r = 1.20$

Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Jeklo: S 235

Podpore in uklonska polja okoli osi y

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

Podpore in uklonska polja okoli osi z

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

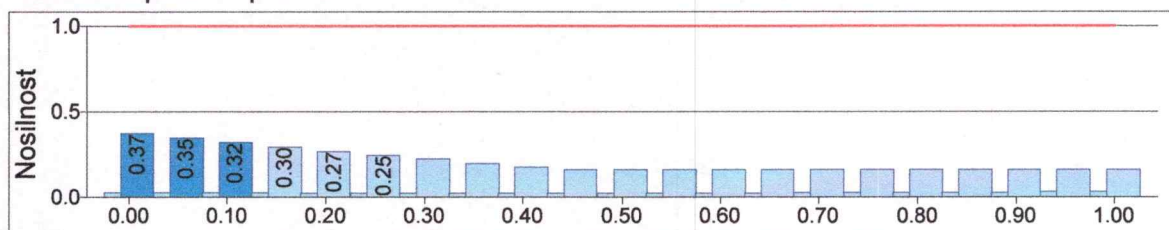
Podpore in uklonska polja za bočno zvrnitev

Polje	x začetek	x konec	uklonski koef.
1	0.000	1.000	1.000

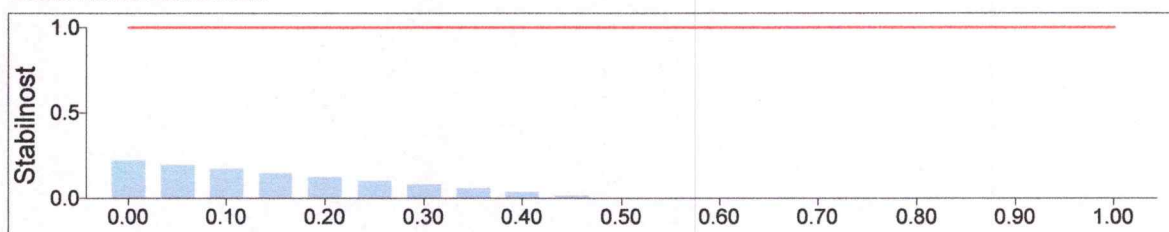
Relativna lega obtežbe za BZ je -1.00 (neugodno).

Mejno stanje nosilnosti

Nosilnosti prečnih prerezov



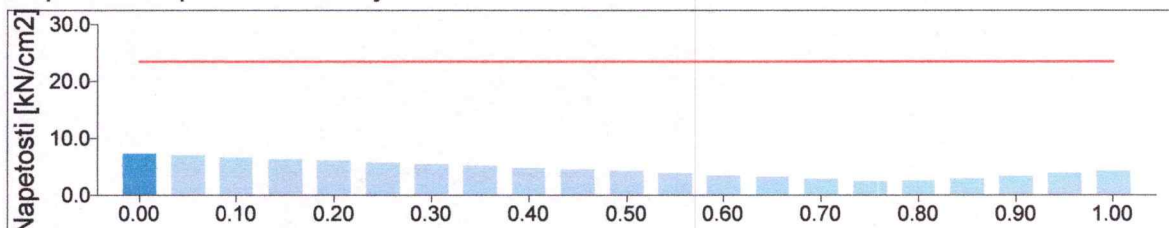
Stabilnost elementa



Element je bil razvrščen v razred 1. Vse kontrole stabilnosti so bile uspešne. Merodajna kontrola je kontrola bočne zvrnitve $[0.220 \leq 1.0]$ za obtežni primer '[U] G+Q'.

Mejno stanje uporabnosti

Napetosti v uporabnem stanju

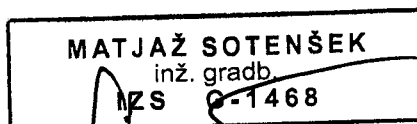


**POZ 4-A.B. TOČKOVNI TEMELJ
B/H/D=300/120/120 CM; TEMELJNI NASTAVEK
120/120/200 CM ; BSt500; C25/30, XC2**

Projekt: Ureditev športnega parka NK Litija-2 faza

Investitor: Občina Litija, Jerebova ulica 14, 1270 Litija

Avtor: Sotenšek Matjaž, inž.gr.



Komentar:

OPOMBA: Vsa dela lahko izvajajo le pooblašчени izvajalci. Vsako spremembo v projektu je potrebno evidentirati v gradbeni dnevnik in potrditi s strani pooblaščenega organizavije(Nadzor).

POZ 4-A.B. TOČKOVNI TEMELJ B/H/D=300/120/120 CM; TEMELJNI NASTAVEK 120/120/200 CM ; BSt500; C25/30, XC2

Shema konstrukcije

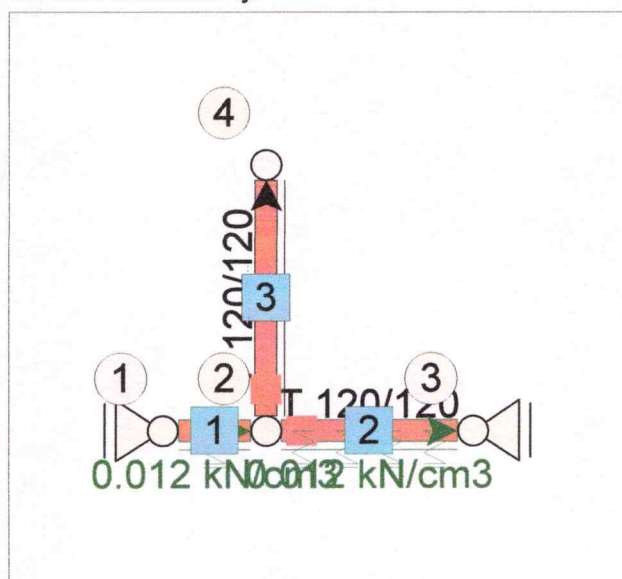


Tabela vozlišč

Index	X koor. [cm]	Z koor. [cm]	Podpore
1:	0.00000	0.00000	Ux
2:	100.00	0.00000	-

Index	X koor. [cm]	Z koor. [cm]	Podpore
3:	300.00	0.00000	Ux
4:	100.00	-260.00	-

Tabela elementov

Index	Tip	Voz. 1	Voz. 2	Prečni prerez	Material	Dolžina [cm]	Spr. 1	Spr. 2
1:	nelin.	1:	2:	1: T 120/120	1: C 25/30	100.00	-	-
2:	nelin.	2:	3:	1: T 120/120	1: C 25/30	200.00	-	-
3:	nelin.	2:	4:	1: T 120/120	1: C 25/30	260.00	-	-

Element na elastični podlagi

Index	Lega	Modul reakcije tal [kN/cm ³]
1:	Pozitivna stran	0.012000
2:	Pozitivna stran	0.012000

Tabela materialov

Index	Elastični modul [kN/cm ²]	Koef. termičnega raztezka [1/d°C]
1: C 25/30	3050.0	1.0000E-05
2: BSt500	20000.	1.2000E-05

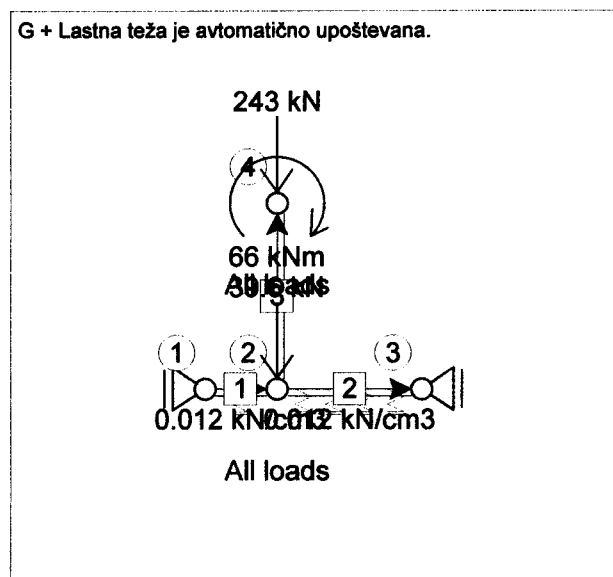
Tabela prečnih prerezov

Index	A [cm ²]	I _y [cm ⁴]	Višina [cm]
1: T 120/120	14400.	1.7280E+07	120.00

Obtežni vplivi in obtežni primeri

Obtežni primer	Mejno stanje	G	Q
G+Q	nosilnosti	1.35	1.50
G+Q/ser	uporabnosti	1.00	1.00

A1 : G

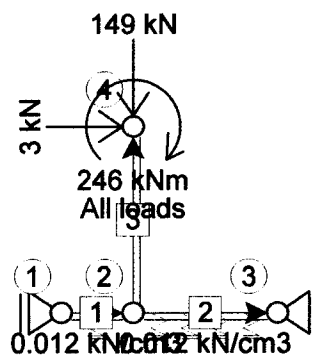


A1: G: Vozliščna obtežba

Voz.	F _x [kN]	F _z [kN]	M _y [kNm]	Ime
2:	0.00000	39.600	0.00000	G
4:	0.00000	243.00	-66.000	G

A2 : Q

Q

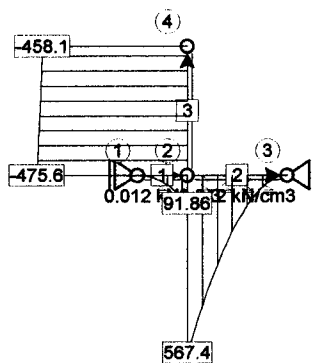


A2: Q: Vozliščna obtežba

Voz.	Fx [kN]	Fz [kN]	My [kNm]	Ime
4:	3.0000	149.00	-246.00	Q

LC1: G+Q: Upogibni moment My

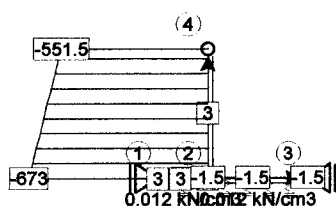
1.35 G + 1.50 Q



Enote: kNm

LC1: G+Q: Osnova sila Fx

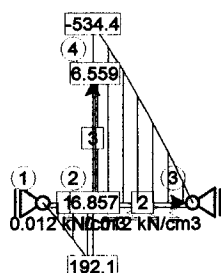
1.35 G + 1.50 Q



Enote: kN

LC1: G+Q: Prečna sila Fz

1.35 G + 1.50 Q



Enote: kN

LC1: G+Q: Vozlišni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
1:	-	1.4834	-0.20099
2:	0.00000	1.8340	-0.20067

Voz.	ux [cm]	uz [cm]	fiy [deg]
3:	-	2.5235	-0.19643
4:	0.94071	1.8394	-0.21386

LC1: G+Q: Reakcije

Voz.	Fx [kN]	Fz [kN]	My [kNm]
1:	-3.0000	-	-
3:	-1.5000	-	-

LC1: G+Q: Element 1:

Lega	Fx [kN]	Fz [kN]	My [kNm]	Tlak [kPa]
0.00	3.0000	0.00000	0.00000	178.00
0.05	3.0000	8.4072	0.20965	180.11
0.10	3.0000	16.941	0.84283	182.21
0.15	3.0000	25.601	1.9058	184.32
0.20	3.0000	34.387	3.4050	186.42
0.25	3.0000	43.299	5.3466	188.53
0.30	3.0000	52.338	7.7370	190.63
0.35	3.0000	61.503	10.582	192.74
0.40	3.0000	70.794	13.889	194.84
0.45	3.0000	80.211	17.664	196.95
0.50	3.0000	89.755	21.913	199.05

Lega	Fx [kN]	Fz [kN]	My [kNm]	Tlak [kPa]
0.55	3.0000	99.425	26.642	201.16
0.60	3.0000	109.22	31.857	203.26
0.65	3.0000	119.14	37.566	205.36
0.70	3.0000	129.19	43.774	207.47
0.75	3.0000	139.37	50.487	209.57
0.80	3.0000	149.67	57.713	211.67
0.85	3.0000	160.10	65.456	213.78
0.90	3.0000	170.65	73.724	215.88
0.95	3.0000	181.33	82.523	217.98
1.00	3.0000	192.14	91.859	220.08

LC1: G+Q: Element 2:

Lega	Fx [kN]	Fz [kN]	My [kNm]	Tlak [kPa]
0.00	-1.5000	-534.35	567.41	220.08
0.05	-1.5000	-512.37	515.07	224.28
0.10	-1.5000	-489.87	464.96	228.46
0.15	-1.5000	-466.88	417.11	232.64
0.20	-1.5000	-443.38	371.60	236.80
0.25	-1.5000	-419.39	328.45	240.96
0.30	-1.5000	-394.90	287.74	245.11
0.35	-1.5000	-369.91	249.49	249.25
0.40	-1.5000	-344.42	213.77	253.39
0.45	-1.5000	-318.44	180.62	257.52
0.50	-1.5000	-291.96	150.10	261.65

Lega	Fx [kN]	Fz [kN]	My [kNm]	Tlak [kPa]
0.55	-1.5000	-264.99	122.25	265.77
0.60	-1.5000	-237.52	97.117	269.89
0.65	-1.5000	-209.56	74.758	274.01
0.70	-1.5000	-181.11	55.221	278.13
0.75	-1.5000	-152.16	38.553	282.24
0.80	-1.5000	-122.71	24.806	286.36
0.85	-1.5000	-92.775	14.027	290.47
0.90	-1.5000	-62.344	6.2673	294.59
0.95	-1.5000	-31.419	1.5751	298.70
1.00	-1.5000	0.00000	0.00000	302.82

LC1: G+Q: Element 3:

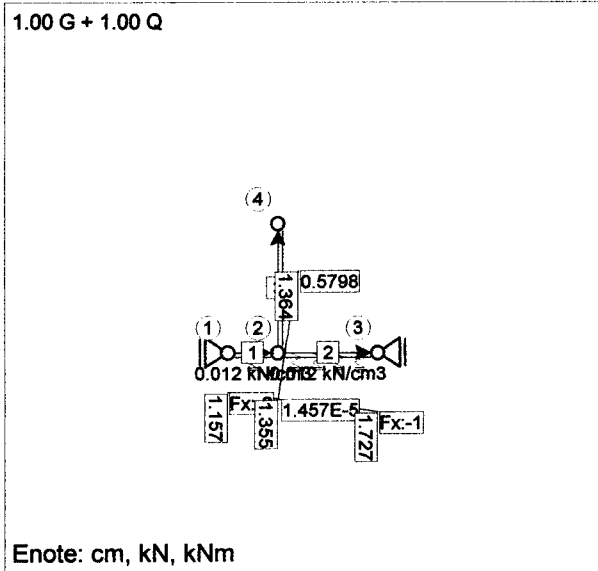
Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-673.01	6.8571	-475.55
0.05	-666.94	6.8437	-474.66
0.10	-660.86	6.8300	-473.77
0.15	-654.79	6.8163	-472.89
0.20	-648.71	6.8023	-472.00
0.25	-642.64	6.7883	-471.12
0.30	-636.57	6.7740	-470.24
0.35	-630.49	6.7596	-469.36
0.40	-624.42	6.7451	-468.48
0.45	-618.34	6.7304	-467.60

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.50	-612.27	6.7155	-466.73
0.55	-606.20	6.7005	-465.86
0.60	-600.12	6.6854	-464.99
0.65	-594.05	6.6701	-464.12
0.70	-587.97	6.6546	-463.25
0.75	-581.90	6.6390	-462.39
0.80	-575.83	6.6232	-461.53
0.85	-569.75	6.6073	-460.67
0.90	-563.68	6.5912	-459.81
0.95	-557.60	6.5750	-458.95

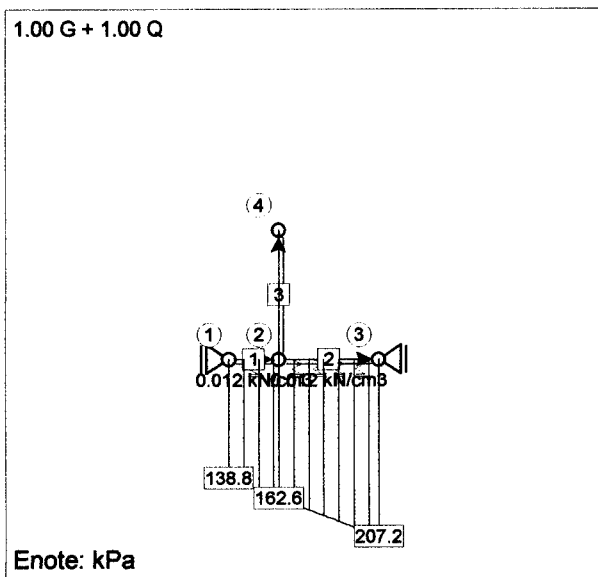
LC1: G+Q: Element 3:

Lega	Fx [kN]	Fz [kN]	My [kNm]
1.00	-551.53	6.5587	-458.10

LC2: G+Q/ser: Pomiki in reakcije

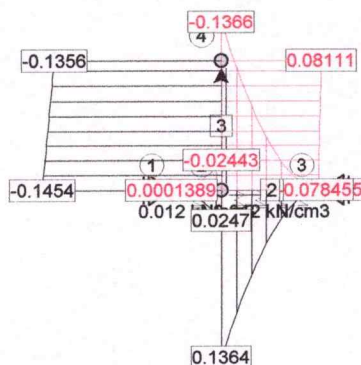


LC2: G+Q/ser: Kontaktni tlak



LC2: G+Q/ser: Napetost

1.00 G + 1.00 Q



Enote: kN/cm2

LC2: G+Q/ser: Vozlišni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
1:	-	1.1565	-0.11415
2:	1.4572E-05	1.3554	-0.11334

Voz.	ux [cm]	uz [cm]	fiy [deg]
3:	-	1.7268	-0.10402
4:	0.57978	1.3643	-0.14203

LC2: G+Q/ser: Reakcije

Voz.	Fx [kN]	Fz [kN]	My [kNm]
1:	-2.0000	-	-
3:	-1.0000	-	-

LC2: G+Q/ser: Element 1:

Lega	Fx [kN]	Fz [kN]	My [kNm]	Tlak [kPa]
0.00	2.0000	0.00000	0.00000	138.78
0.05	2.0000	6.6324	0.16551	139.98
0.10	2.0000	13.336	0.66443	141.17
0.15	2.0000	20.112	1.5003	142.37
0.20	2.0000	26.960	2.6768	143.56
0.25	2.0000	33.879	4.1975	144.76
0.30	2.0000	40.870	6.0659	145.95
0.35	2.0000	47.933	8.2857	147.15
0.40	2.0000	55.067	10.860	148.34
0.45	2.0000	62.273	13.794	149.54
0.50	2.0000	69.551	17.089	150.73

Lega	Fx [kN]	Fz [kN]	My [kNm]	Tlak [kPa]
0.55	2.0000	76.900	20.750	151.93
0.60	2.0000	84.321	24.780	153.12
0.65	2.0000	91.814	29.183	154.32
0.70	2.0000	99.378	33.963	155.51
0.75	2.0000	107.01	39.122	156.70
0.80	2.0000	114.72	44.665	157.89
0.85	2.0000	122.50	50.595	159.08
0.90	2.0000	130.35	56.916	160.27
0.95	2.0000	138.27	63.632	161.46
1.00	2.0000	146.26	70.745	162.65

LC2: G+Q/ser: Element 2:

Lega	Fx [kN]	Fz [kN]	My [kNm]	Tlak [kPa]
0.00	-1.0000	-375.32	393.07	162.65
0.05	-1.0000	-359.12	356.34	165.01
0.10	-1.0000	-342.64	321.25	167.34
0.15	-1.0000	-325.88	287.82	169.65
0.20	-1.0000	-308.85	256.09	171.94
0.25	-1.0000	-291.54	226.06	174.21
0.30	-1.0000	-273.96	197.79	176.47
0.35	-1.0000	-256.11	171.28	178.71
0.40	-1.0000	-237.99	146.57	180.94
0.45	-1.0000	-219.61	123.69	183.15
0.50	-1.0000	-200.96	102.66	185.36

Lega	Fx [kN]	Fz [kN]	My [kNm]	Tlak [kPa]
0.55	-1.0000	-182.04	83.510	187.56
0.60	-1.0000	-162.86	66.262	189.76
0.65	-1.0000	-143.42	50.946	191.95
0.70	-1.0000	-123.72	37.587	194.13
0.75	-1.0000	-103.75	26.211	196.31
0.80	-1.0000	-83.526	16.845	198.49
0.85	-1.0000	-63.037	9.5143	200.67
0.90	-1.0000	-42.286	4.2460	202.85
0.95	-1.0000	-21.274	1.0659	205.03
1.00	-1.0000	0.00000	0.00000	207.21

LC2: G+Q/ser: Element 3:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-481.98	3.9535	-322.32
0.05	-477.48	3.9567	-321.81
0.10	-472.98	3.9597	-321.29
0.15	-468.48	3.9624	-320.78
0.20	-463.98	3.9649	-320.26
0.25	-459.48	3.9672	-319.75
0.30	-454.98	3.9692	-319.23
0.35	-450.48	3.9709	-318.72
0.40	-445.98	3.9725	-318.20
0.45	-441.48	3.9737	-317.68
0.50	-436.98	3.9748	-317.17

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	-432.49	3.9755	-316.65
0.60	-427.99	3.9761	-316.13
0.65	-423.49	3.9764	-315.62
0.70	-418.99	3.9764	-315.10
0.75	-414.49	3.9762	-314.58
0.80	-409.99	3.9758	-314.07
0.85	-405.49	3.9752	-313.55
0.90	-400.99	3.9743	-313.03
0.95	-396.49	3.9731	-312.52
1.00	-391.99	3.9717	-312.00

Rezultati za element 1 (Preverjeno po EC2):

Prečni prerez [cm]

[T 120/120]

h = 120.00

b = 120.00

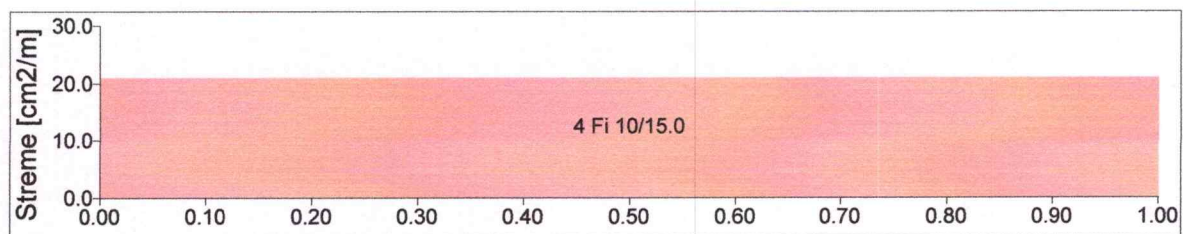
Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Beton: C 25/30

Armatura: BSt500

Stremenska armatura

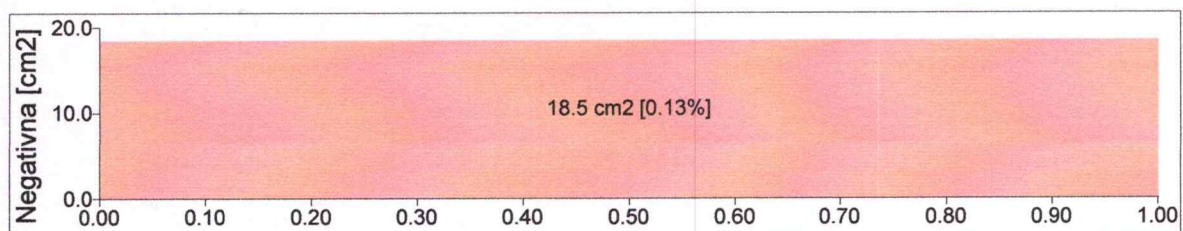
n	Fi [mm]	e [cm]	xs	xe
4	10.000	15.000	0.00	1.00



Negativna armatura

Vrstica	n	Fi [mm]	xs	xe
1	12	14.000	0.00	1.00

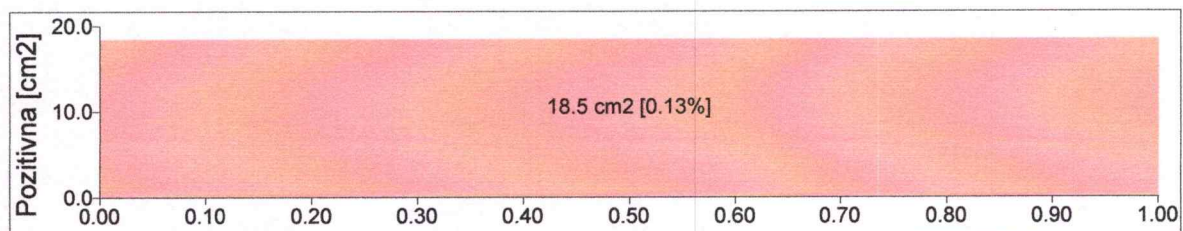
Zaščitni sloj znaša 3.0000 cm.



Pozitivna armatura

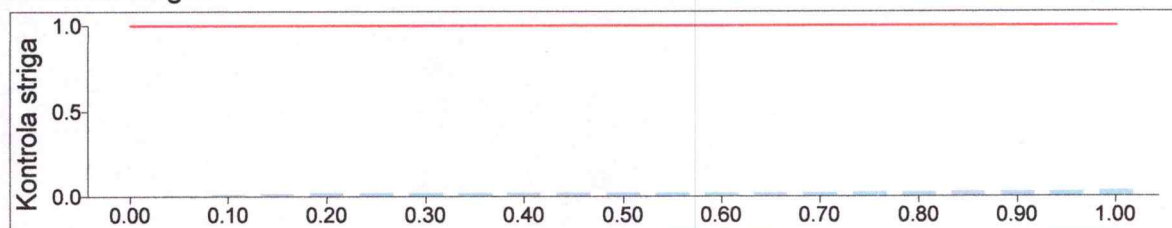
Vrstica	n	Fi [mm]	xs	xe
1	12	14.000	0.00	1.00

Zaščitni sloj znaša 3.0000 cm.

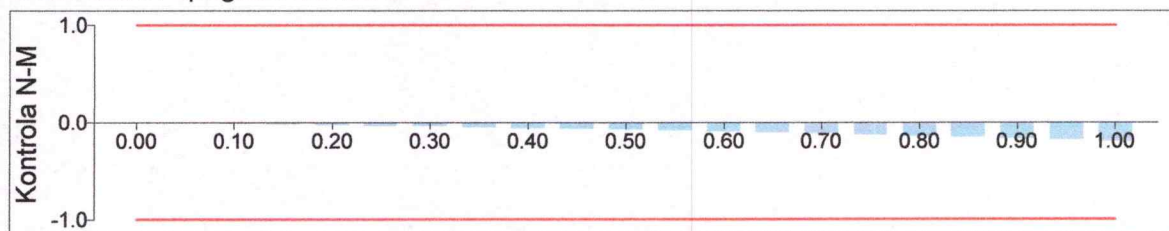


Mejno stanje nosilnosti

Kontrola striga

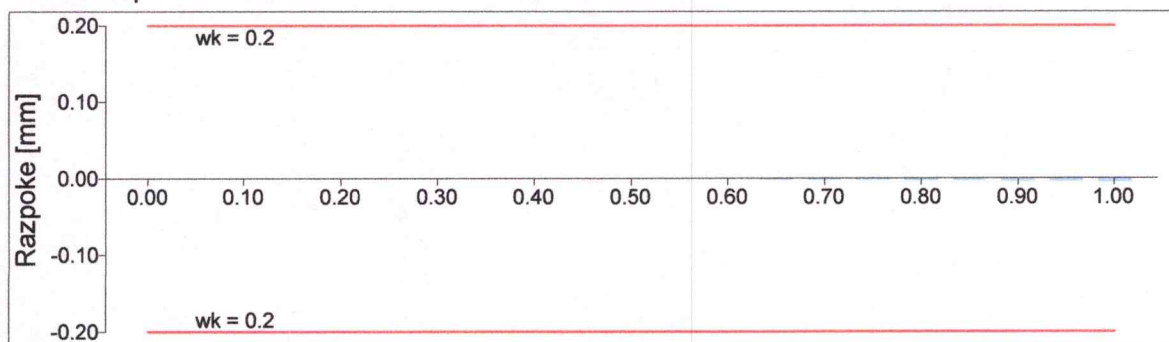


Oсна сила in upogibni moment



Mejno stanje uporabnosti

Širina razpok



Rezultati za element 2 (Preverjeno po EC2):

Prečni prerez [cm]

[T 120/120]

$h = 120.00$

$b = 120.00$

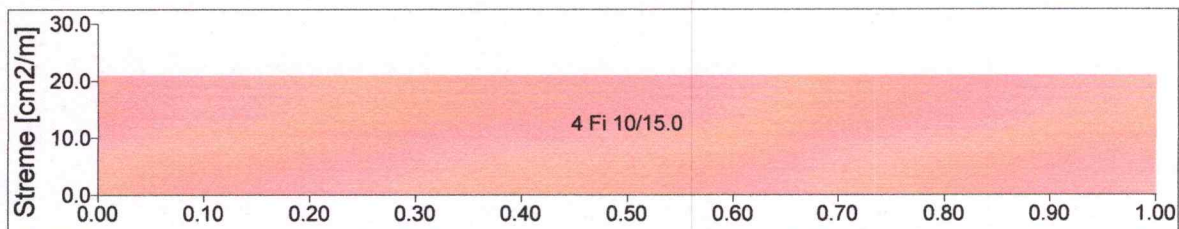
Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Beton: C 25/30

Armatura: BSt500

Stremenska armatura

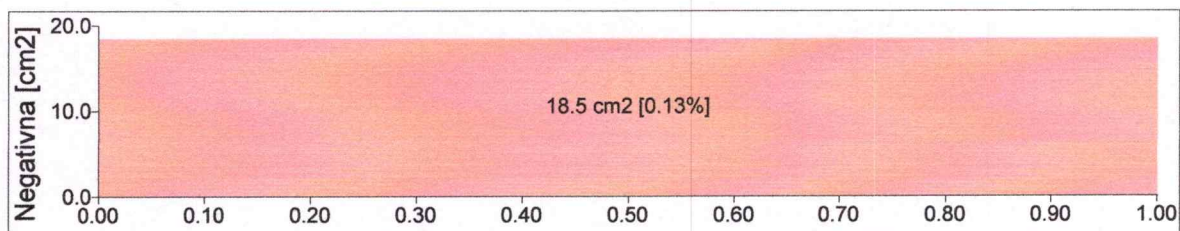
n	Fi [mm]	e [cm]	xs	xe
4	10.000	15.000	0.00	1.00



Negativna armatura

Vrstica	n	Fi [mm]	xs	xe
1	12	14.000	0.00	1.00

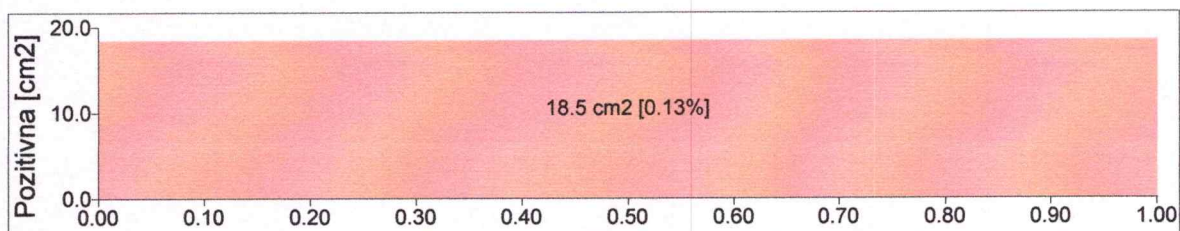
Zaščitni sloj znaša 3.0000 cm.



Pozitivna armatura

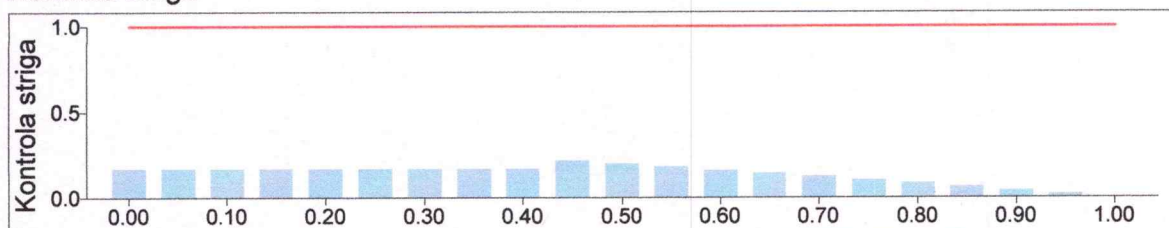
Vrstica	n	Fi [mm]	xs	xe
1	12	14.000	0.00	1.00

Zaščitni sloj znaša 3.0000 cm.

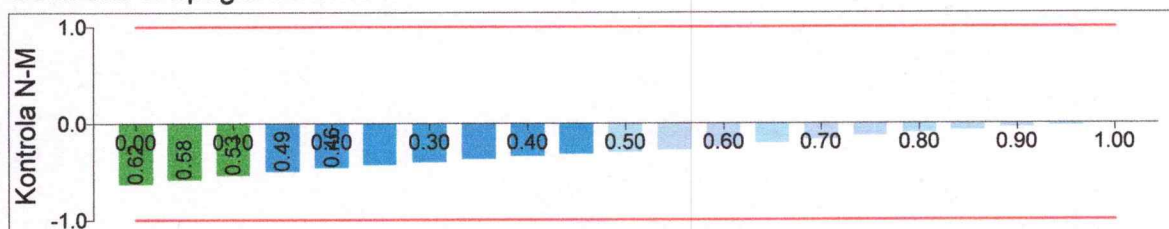


Mejno stanje nosilnosti

Kontrola striga

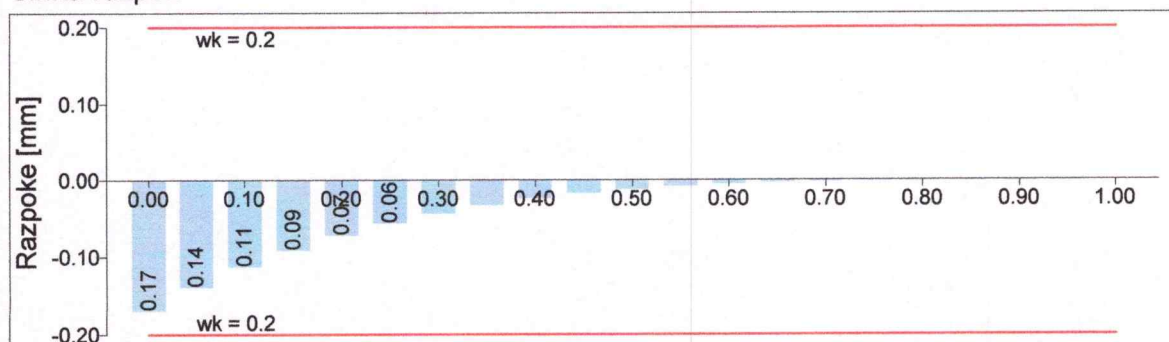


Osna sila in upogibni moment



Mejno stanje uporabnosti

Širina razpok



Rezultati za element 3 (Preverjeno po EC2):

Prečni prerez [cm]

[T 120/120]

$h = 120.00$

$b = 120.00$

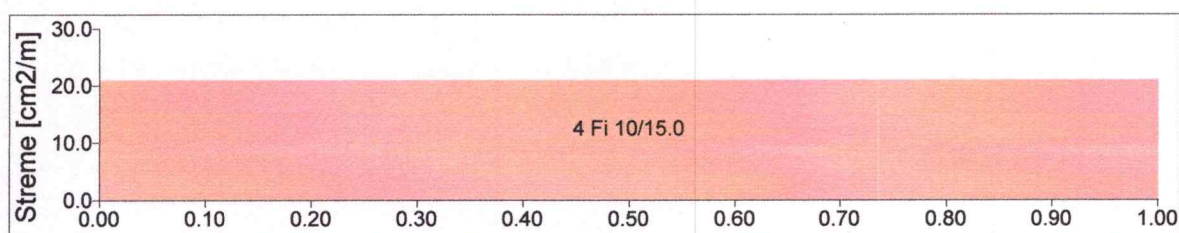
Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Beton: C 25/30

Armatura: BSt500

Stremenska armatura

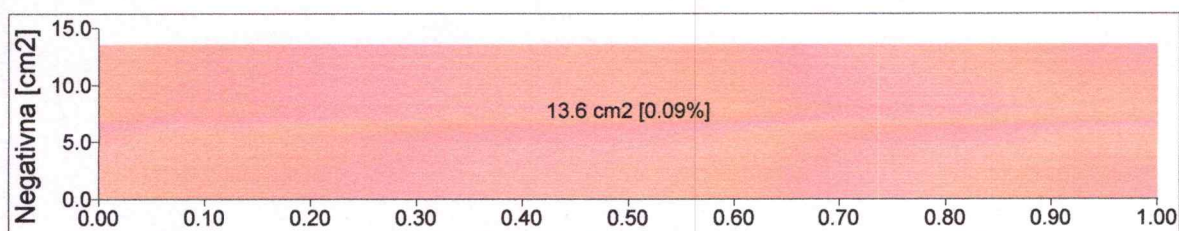
n	Fi [mm]	e [cm]	xs	xe
4	10.000	15.000	0.00	1.00



Negativna armatura

Vrstica	n	Fi [mm]	xs	xe
1	12	12.000	0.00	1.00

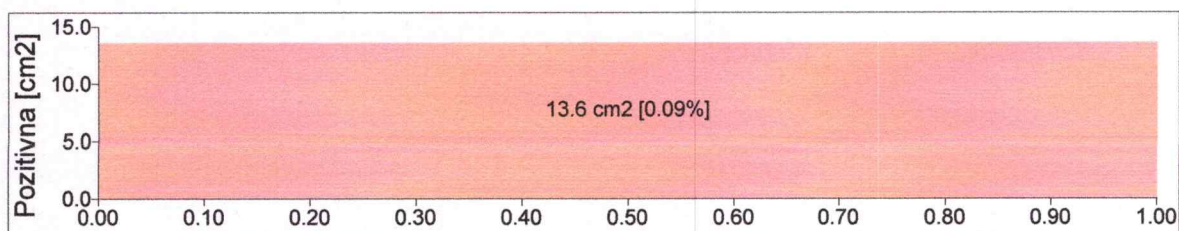
Zaščitni sloj znaša 3.0000 cm.



Pozitivna armatura

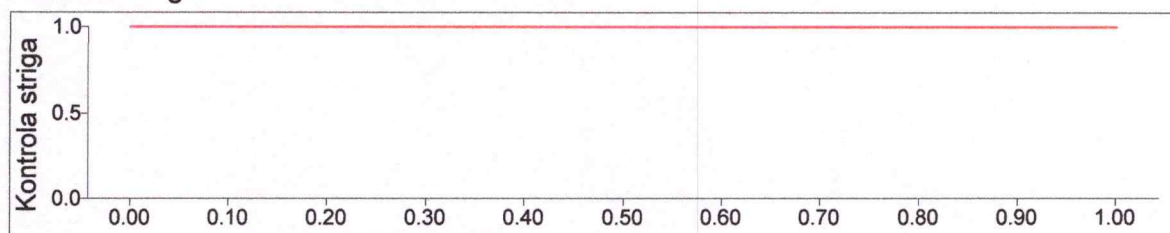
Vrstica	n	Fi [mm]	xs	xe
1	12	12.000	0.00	1.00

Zaščitni sloj znaša 3.0000 cm.

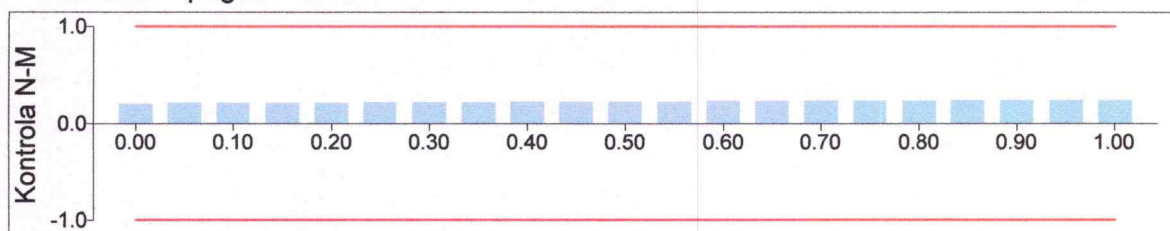


Mejno stanje nosilnosti

Kontrola striga

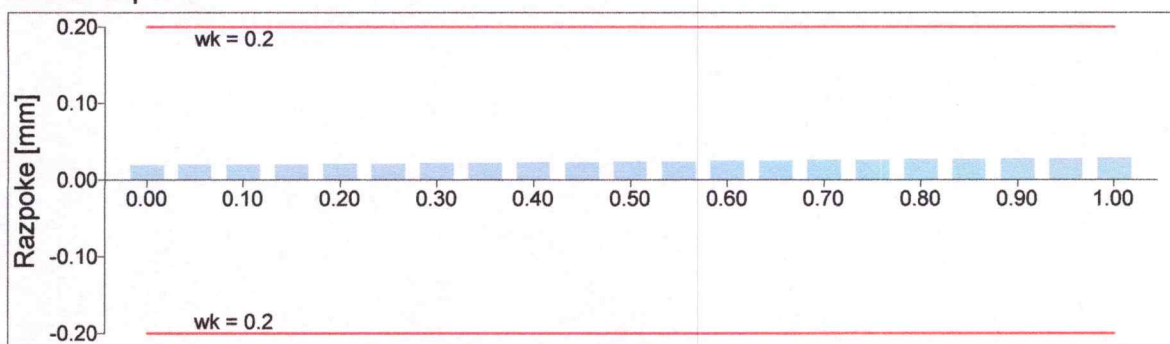


Oсна sila in upogibni moment



Mejno stanje uporabnosti

Širina razpok



POZ 5-OPORNI ZID D=25,0 CM; NOSILEC TRIBUN D=25,0 CM; BSt500; C25/30; XC2

Shema konstrukcije

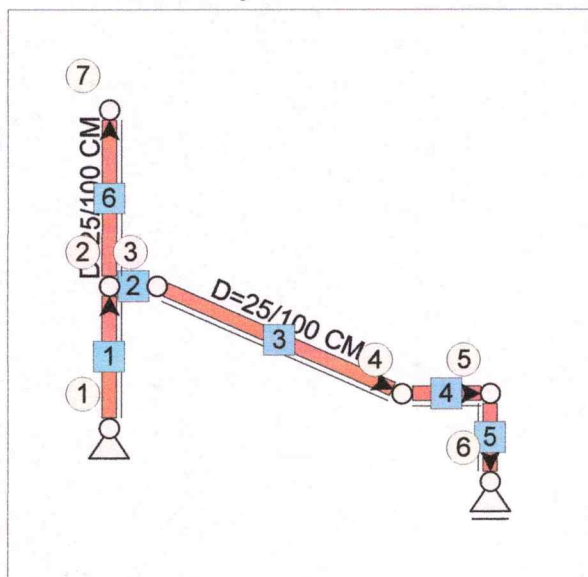


Tabela vozlišč

Index	X koor. [cm]	Z koor. [cm]	Podpore
1:	0.00000	-80.000	Ux Uz
2:	0.00000	-290.00	-
3:	70.000	-290.00	-
4:	430.00	-130.00	-

Index	X koor. [cm]	Z koor. [cm]	Podpore
5:	560.00	-130.00	-
6:	560.00	0.00000	Uz
7:	0.00000	-550.00	-

Tabela elementov

Index	Tip	Voz. 1	Voz. 2	Prečni prerez	Material	Dolžina [cm]	Spr. 1	Spr. 2
1:	nelin.	1:	2:	1: D=25/100 CM	1: C 25/30	210.00	-	-
2:	nelin.	2:	3:	1: D=25/100 CM	1: C 25/30	70.000	-	-
3:	nelin.	3:	4:	1: D=25/100 CM	1: C 25/30	393.95	-	-
4:	nelin.	4:	5:	1: D=25/100 CM	1: C 25/30	130.00	-	-
5:	nelin.	5:	6:	1: D=25/100 CM	1: C 25/30	130.00	-	-
6:	nelin.	2:	7:	1: D=25/100 CM	1: C 25/30	260.00	-	-

Tabela materialov

Index	Elastični modul [kN/cm ²]	Koef. termičnega raztezka [1/d°C]
1: C 25/30	3050.0	1.0000E-05
2: BSt500	20000.	1.2000E-05

Tabela prečnih prerezov

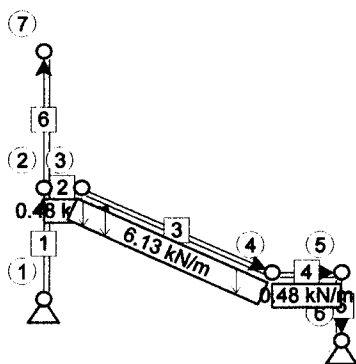
Index	A [cm ²]	I _y [cm ⁴]	Višina [cm]
1: D=25/100 CM	2500.0	1.3021E+05	25.000

Obtežni vplivi in obtežni primeri

Obtežni primer	Mejno stanje	G	Q
G+Q	nosilnosti	1.35	1.50
G+Q/ser	uporabnosti	1.00	1.00

A1 : G

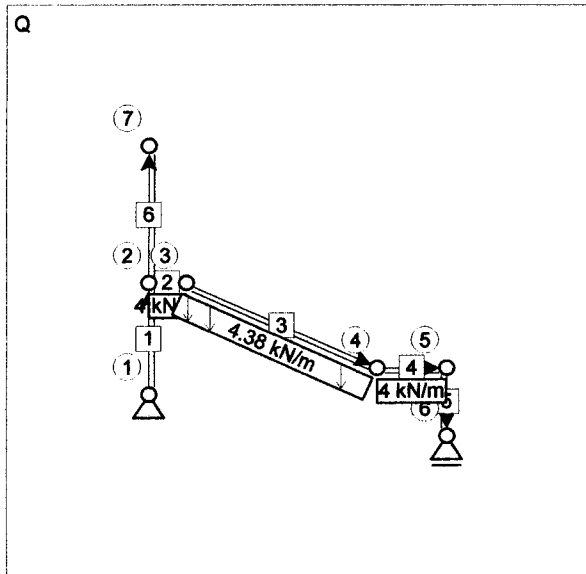
G + Lastna teža je avtomatično upoštevana.



A1: G: Enakomerna zvezna obtežba elementov

Element	F _{x s} [kN/m]	F _{x e} [kN/m]	F _{z s} [kN/m]	F _{z e} [kN/m]	M _{y s} [kNm/m]	M _{y e} [kNm/m]	Smer	Začetek	Konec	Lega
2:	0.00000	0.00000	0.48000	0.48000	0.00000	0.00000	Globalni	0.00	1.00	Lokalni
3:	0.00000	0.00000	6.1300	6.1300	0.00000	0.00000	Globalni	0.00	1.00	Lokalni
4:	0.00000	0.00000	0.48000	0.48000	0.00000	0.00000	Globalni	0.00	1.00	Lokalni

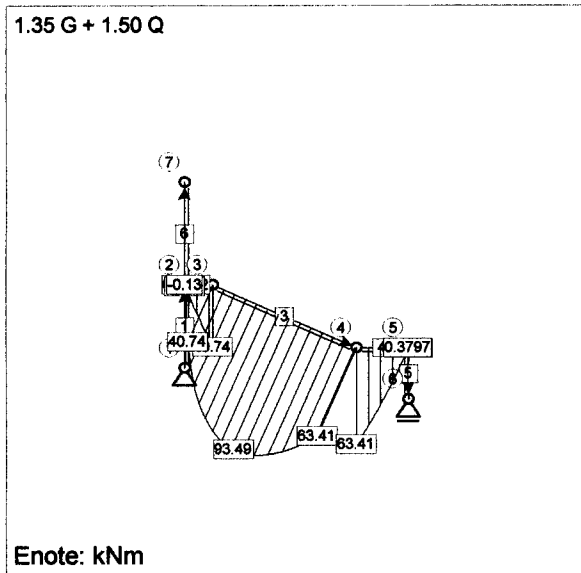
A2 : Q



A2: Q: Enakomerna zvezna obtežba elementov

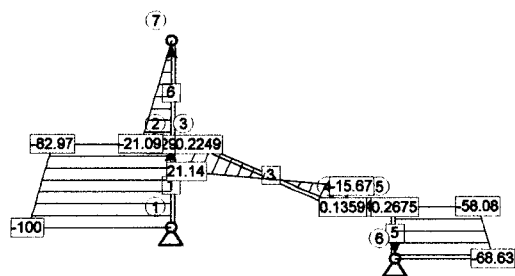
Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Smer	Začetek	Konec	Lega
2:	0.00000	0.00000	4.0000	4.0000	0.00000	0.00000	Lokalni	0.00	1.00	Lokalni
3:	0.00000	0.00000	4.3800	4.3800	0.00000	0.00000	Globalni	0.00	1.00	Lokalni
4:	0.00000	0.00000	4.0000	4.0000	0.00000	0.00000	Lokalni	0.00	1.00	Lokalni

LC1: G+Q: Upogibni moment My



LC1: G+Q: Osnova sila Fx

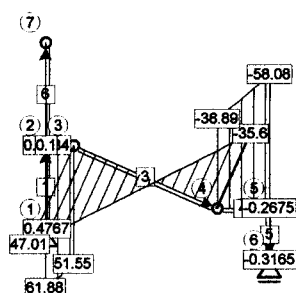
1.35 G + 1.50 Q



Enote: kN

LC1: G+Q: Prečna sila Fz

1.35 G + 1.50 Q



Enote: kN

LC1: G+Q: Vozlišni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
1:	-	-	-0.27311
2:	0.99920	0.0048968	-0.27168
3:	0.99846	0.32763	-0.24997
4:	0.89787	0.55125	0.20016

Voz.	ux [cm]	uz [cm]	fiy [deg]
5:	0.89671	0.0024614	0.26388
6:	1.4960	-	0.26424
7:	2.2326	0.0081818	-0.27184

LC1: G+Q: Reakcije

Voz.	Fx [kN]	Fz [kN]	My [kNm]
1:	-0.00014528	-99.976	-
6:	-	-68.616	-

LC1: G+Q: Element 1:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-100.00	0.47668	0.00000
0.05	-99.151	0.47261	0.049837
0.10	-98.299	0.46853	0.099246
0.15	-97.447	0.46444	0.14823
0.20	-96.596	0.46034	0.19678
0.25	-95.744	0.45622	0.24490
0.30	-94.892	0.45209	0.29258
0.35	-94.041	0.44796	0.33983
0.40	-93.189	0.44381	0.38665
0.45	-92.337	0.43966	0.43303
0.50	-91.485	0.43549	0.47898

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	-90.634	0.43132	0.52448
0.60	-89.782	0.42713	0.56955
0.65	-88.930	0.42294	0.61418
0.70	-88.079	0.41874	0.65837
0.75	-87.227	0.41454	0.70212
0.80	-86.375	0.41032	0.74542
0.85	-85.523	0.40610	0.78828
0.90	-84.672	0.40188	0.83070
0.95	-83.820	0.39765	0.87268
1.00	-82.968	0.39341	0.91421

LC1: G+Q: Element 2:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.29341	61.878	1.0443
0.05	0.29084	61.362	3.2010
0.10	0.28817	60.845	5.3396
0.15	0.28538	60.329	7.4601
0.20	0.28249	59.812	9.5626
0.25	0.27949	59.295	11.647
0.30	0.27640	58.779	13.713
0.35	0.27321	58.262	15.761
0.40	0.26994	57.746	17.792
0.45	0.26658	57.229	19.804
0.50	0.26313	56.713	21.798

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.25960	56.196	23.774
0.60	0.25600	55.679	25.731
0.65	0.25233	55.163	27.671
0.70	0.24859	54.646	29.593
0.75	0.24478	54.130	31.496
0.80	0.24091	53.613	33.382
0.85	0.23698	53.096	35.249
0.90	0.23300	52.580	37.099
0.95	0.22897	52.063	38.930
1.00	0.22489	51.547	40.743

LC1: G+Q: Element 3:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	21.141	47.013	40.744
0.05	19.276	42.893	49.598
0.10	17.412	38.773	57.641
0.15	15.549	34.652	64.872
0.20	13.688	30.531	71.292
0.25	11.828	26.409	76.900
0.30	9.9709	22.286	81.696
0.35	8.1166	18.162	85.680
0.40	6.2655	14.036	88.851
0.45	4.4178	9.9087	91.210

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.50	2.5738	5.7798	92.755
0.55	0.73346	1.6492	93.486
0.60	-1.1030	-2.4832	93.404
0.65	-2.9357	-6.6172	92.508
0.70	-4.7646	-10.753	90.797
0.75	-6.5899	-14.890	88.272
0.80	-8.4118	-19.029	84.931
0.85	-10.231	-23.169	80.775
0.90	-12.047	-27.311	75.804
0.95	-13.860	-31.453	70.016

LC1: G+Q: Element 3:

Lega	Fx [kN]	Fz [kN]	My [kNm]
1.00	-15.672	-35.596	63.412

LC1: G+Q: Element 4:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.13587	-38.893	63.412
0.05	0.14328	-39.853	60.853
0.10	0.15070	-40.812	58.232
0.15	0.15813	-41.771	55.548
0.20	0.16556	-42.731	52.801
0.25	0.17295	-43.690	49.993
0.30	0.18029	-44.649	47.122
0.35	0.18758	-45.609	44.188
0.40	0.19478	-46.568	41.193
0.45	0.20188	-47.527	38.134
0.50	0.20885	-48.487	35.014

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.21569	-49.446	31.831
0.60	0.22237	-50.405	28.586
0.65	0.22887	-51.365	25.278
0.70	0.23516	-52.324	21.909
0.75	0.24123	-53.284	18.476
0.80	0.24706	-54.243	14.982
0.85	0.25263	-55.202	11.425
0.90	0.25790	-56.162	7.8054
0.95	0.26286	-57.121	4.1237
1.00	0.26749	-58.080	0.37966

LC1: G+Q: Element 5:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-58.080	-0.26749	0.37966
0.05	-58.608	-0.26996	0.36220
0.10	-59.135	-0.27242	0.34457
0.15	-59.662	-0.27488	0.32678
0.20	-60.189	-0.27734	0.30883
0.25	-60.717	-0.27980	0.29073
0.30	-61.244	-0.28226	0.27246
0.35	-61.771	-0.28472	0.25403
0.40	-62.298	-0.28717	0.23545
0.45	-62.826	-0.28963	0.21670
0.50	-63.353	-0.29208	0.19780

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	-63.880	-0.29453	0.17873
0.60	-64.407	-0.29698	0.15951
0.65	-64.935	-0.29942	0.14012
0.70	-65.462	-0.30187	0.12058
0.75	-65.989	-0.30431	0.10088
0.80	-66.516	-0.30675	0.081023
0.85	-67.044	-0.30919	0.061005
0.90	-67.571	-0.31163	0.040828
0.95	-68.098	-0.31407	0.020493
1.00	-68.625	-0.31650	0.00000

LC1: G+Q: Element 6:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-21.090	0.10000	-0.13005
0.05	-20.036	0.095010	-0.11737
0.10	-18.981	0.090017	-0.10535
0.15	-17.927	0.085022	-0.093970
0.20	-16.872	0.080025	-0.083242
0.25	-15.818	0.075028	-0.073163

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.30	-14.763	0.070029	-0.063734
0.35	-13.709	0.065030	-0.054956
0.40	-12.654	0.060030	-0.046827
0.45	-11.600	0.055029	-0.039348
0.50	-10.545	0.050027	-0.032519
0.55	-9.4905	0.045026	-0.026341

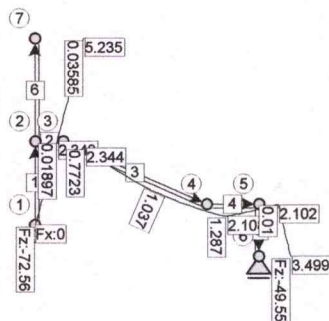
LC1: G+Q: Element 6:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.60	-8.4360	0.040023	-0.020813
0.65	-7.3815	0.035021	-0.015935
0.70	-6.3270	0.030018	-0.011707
0.75	-5.2725	0.025015	-0.0081300
0.80	-4.2180	0.020012	-0.0052032

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.85	-3.1635	0.015009	-0.0029269
0.90	-2.1090	0.010006	-0.0013009
0.95	-1.0545	0.0050031	-0.00032521
1.00	0.00000	0.00000	0.00000

LC2: G+Q/ser: Pomiki in reakcije

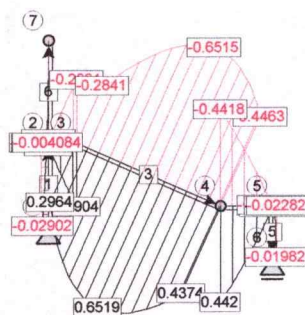
1.00 G + 1.00 Q



Enote: cm, kN, kNm

LC2: G+Q/ser: Napetost

1.00 G + 1.00 Q



Enote: kN/cm²

LC2: G+Q/ser: Vozlišni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
1:	-	-	-0.64331
2:	2.3481	0.018966	-0.63552

Voz.	ux [cm]	uz [cm]	fiy [deg]
3:	2.3440	0.77230	-0.58240
4:	2.1082	1.2872	0.46576

LC2: G+Q/ser: Vozlišni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
5:	2.1019	0.010002	0.61467
6:	3.4995	-	0.61664

Voz.	ux [cm]	uz [cm]	fiy [deg]
7:	5.2350	0.035846	-0.63642

LC2: G+Q/ser: Reakcije

Voz.	Fx [kN]	Fz [kN]	My [kNm]
1:	0.00000	-72.558	-
6:	-	-49.546	-

LC2: G+Q/ser: Element 1:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-72.554	0.81466	0.00000
0.05	-71.923	0.80755	0.085164
0.10	-71.292	0.80039	0.16958
0.15	-70.661	0.79319	0.25324
0.20	-70.030	0.78593	0.33614
0.25	-69.399	0.77863	0.41828
0.30	-68.769	0.77128	0.49965
0.35	-68.138	0.76390	0.58024
0.40	-67.507	0.75647	0.66006
0.45	-66.876	0.74900	0.73909
0.50	-66.245	0.74150	0.81734

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	-65.614	0.73396	0.89480
0.60	-64.983	0.72639	0.97147
0.65	-64.353	0.71879	1.0473
0.70	-63.722	0.71116	1.1224
0.75	-63.091	0.70350	1.1967
0.80	-62.460	0.69582	1.2701
0.85	-61.829	0.68810	1.3428
0.90	-61.198	0.68037	1.4146
0.95	-60.567	0.67261	1.4857
1.00	-59.937	0.66484	1.5559

LC2: G+Q/ser: Element 2:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.49156	44.315	1.7813
0.05	0.48717	43.948	3.3259
0.10	0.48260	43.581	4.8577
0.15	0.47785	43.214	6.3766
0.20	0.47293	42.847	7.8827
0.25	0.46784	42.480	9.3759
0.30	0.46260	42.113	10.856
0.35	0.45720	41.746	12.324
0.40	0.45166	41.379	13.778
0.45	0.44597	41.011	15.220
0.50	0.44015	40.644	16.649

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.43421	40.277	18.065
0.60	0.42814	39.910	19.469
0.65	0.42195	39.543	20.859
0.70	0.41565	39.176	22.237
0.75	0.40925	38.809	23.601
0.80	0.40275	38.442	24.953
0.85	0.39615	38.075	26.292
0.90	0.38946	37.708	27.619
0.95	0.38269	37.341	28.932
1.00	0.37585	36.974	30.232

LC2: G+Q/ser: Element 3:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	15.360	33.635	30.234
0.05	13.992	30.682	36.567
0.10	12.625	27.730	42.319
0.15	11.259	24.776	47.491
0.20	9.8953	21.822	52.080
0.25	8.5354	18.866	56.088
0.30	7.1795	15.908	59.513
0.35	5.8285	12.948	62.355
0.40	4.4828	9.9853	64.614
0.45	3.1429	7.0202	66.289
0.50	1.8091	4.0524	67.379

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.48152	1.0818	67.884
0.60	-0.83964	-1.8917	67.805
0.65	-2.1544	-4.8681	67.139
0.70	-3.4629	-7.8472	65.887
0.75	-4.7654	-10.829	64.048
0.80	-6.0622	-13.813	61.621
0.85	-7.3538	-16.800	58.606
0.90	-8.6407	-19.788	55.003
0.95	-9.9236	-22.778	50.810
1.00	-11.203	-25.770	46.028

LC2: G+Q/ser: Element 4:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.22842	-28.099	46.029
0.05	0.24077	-28.781	44.180
0.10	0.25314	-29.462	42.287
0.15	0.26552	-30.144	40.350
0.20	0.27788	-30.826	38.368
0.25	0.29019	-31.508	36.343
0.30	0.30243	-32.189	34.272
0.35	0.31455	-32.871	32.158
0.40	0.32654	-33.553	29.999
0.45	0.33835	-34.234	27.796
0.50	0.34997	-34.916	25.549

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.36135	-35.598	23.257
0.60	0.37247	-36.279	20.921
0.65	0.38329	-36.961	18.541
0.70	0.39378	-37.643	16.116
0.75	0.40390	-38.324	13.647
0.80	0.41361	-39.006	11.134
0.85	0.42289	-39.688	8.5764
0.90	0.43169	-40.369	5.9745
0.95	0.43998	-41.051	3.3284
1.00	0.44772	-41.733	0.63788

LC2: G+Q/ser: Element 5:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-41.733	-0.44772	0.63788
0.05	-42.123	-0.45205	0.60864
0.10	-42.514	-0.45638	0.57911
0.15	-42.904	-0.46069	0.54931
0.20	-43.295	-0.46501	0.51922
0.25	-43.686	-0.46932	0.48886
0.30	-44.076	-0.47362	0.45821
0.35	-44.467	-0.47792	0.42729
0.40	-44.857	-0.48222	0.39608
0.45	-45.248	-0.48651	0.36460

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.50	-45.638	-0.49079	0.33284
0.55	-46.029	-0.49507	0.30080
0.60	-46.419	-0.49933	0.26848
0.65	-46.810	-0.50360	0.23589
0.70	-47.200	-0.50785	0.20302
0.75	-47.591	-0.51210	0.16987
0.80	-47.981	-0.51634	0.13644
0.85	-48.372	-0.52058	0.10274
0.90	-48.763	-0.52480	0.068771
0.95	-49.153	-0.52902	0.034522

LC2: G+Q/ser: Element 5:

Lega	Fx [kN]	Fz [kN]	My [kNm]
1.00	-49.544	-0.53322	0.00000

LC2: G+Q/ser: Element 6:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	-15.621	0.17328	-0.22545
0.05	-14.840	0.16465	-0.20349
0.10	-14.059	0.15601	-0.18264
0.15	-13.278	0.14737	-0.16293
0.20	-12.497	0.13872	-0.14433
0.25	-11.716	0.13007	-0.12686
0.30	-10.935	0.12141	-0.11051
0.35	-10.154	0.11275	-0.095293
0.40	-9.3729	0.10408	-0.081199
0.45	-8.5918	0.095416	-0.068232
0.50	-7.8107	0.086747	-0.056391

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	-7.0297	0.078076	-0.045678
0.60	-6.2486	0.069403	-0.036092
0.65	-5.4675	0.060730	-0.027633
0.70	-4.6864	0.052055	-0.020302
0.75	-3.9054	0.043380	-0.014099
0.80	-3.1243	0.034704	-0.0090233
0.85	-2.3432	0.026029	-0.0050758
0.90	-1.5621	0.017352	-0.0022560
0.95	-0.78107	0.0086762	-0.00056397
1.00	0.00000	0.00000	0.00000

Rezultati za element 3 (Preverjeno po EC2):

Prečni prerez [cm]

[D=25/100 CM]

h = 25.00

b = 100.00

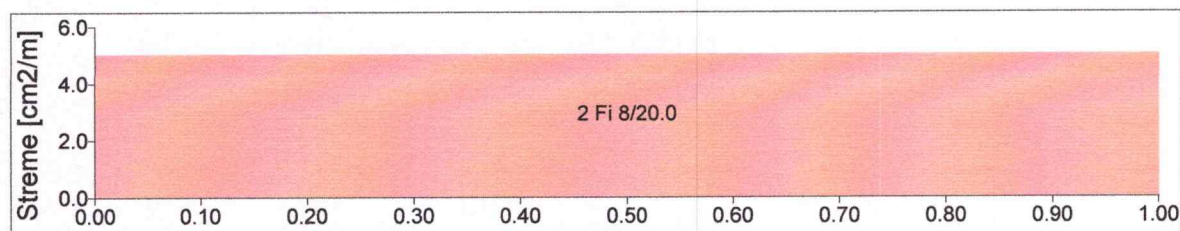
Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Beton: C 25/30

Armatura: BSt500

Stremenska armatura

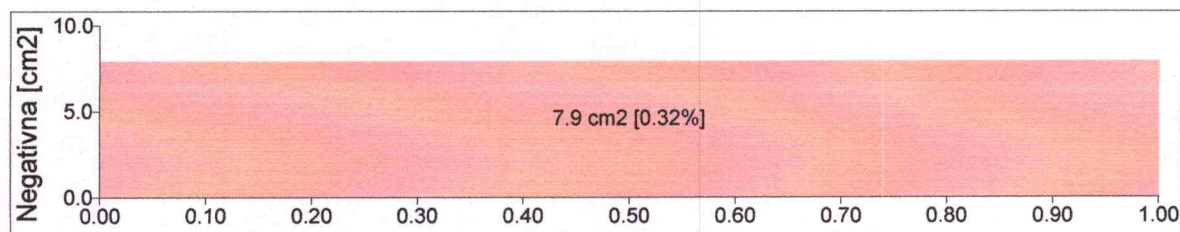
n	Fi [mm]	e [cm]	xs	xe
2	8.0000	20.000	0.00	1.00



Negativna armatura

Vrstica	n	Fi [mm]	xs	xe
1	7	12.000	0.00	1.00

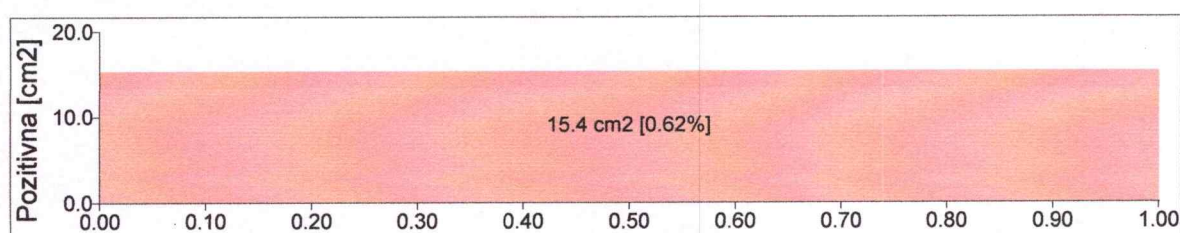
Zaščitni sloj znaša 3.0000 cm.



Pozitivna armatura

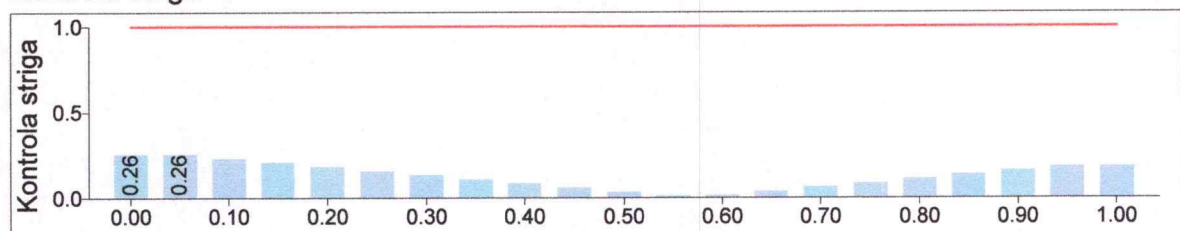
Vrstica	n	Fi [mm]	xs	xe
1	10	14.000	0.00	1.00

Zaščitni sloj znaša 3.0000 cm.

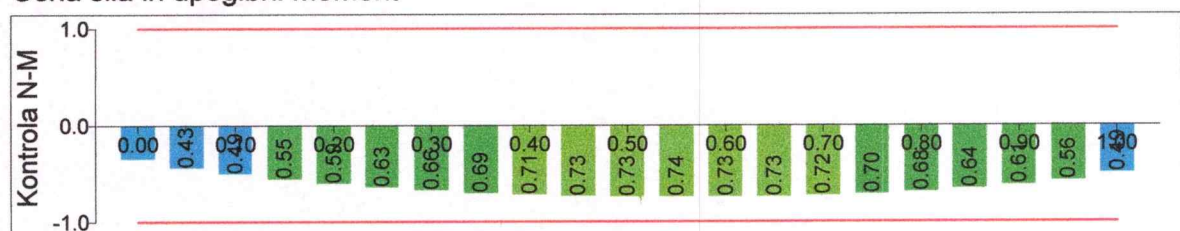


Mejno stanje nosilnosti

Kontrola striga



Osna sila in upogibni moment



Mejno stanje uporabnosti

POZ 6-A.B. NOSILEC B/H=40/100 CM; BSt500; C25/30, XC2

Shema konstrukcije

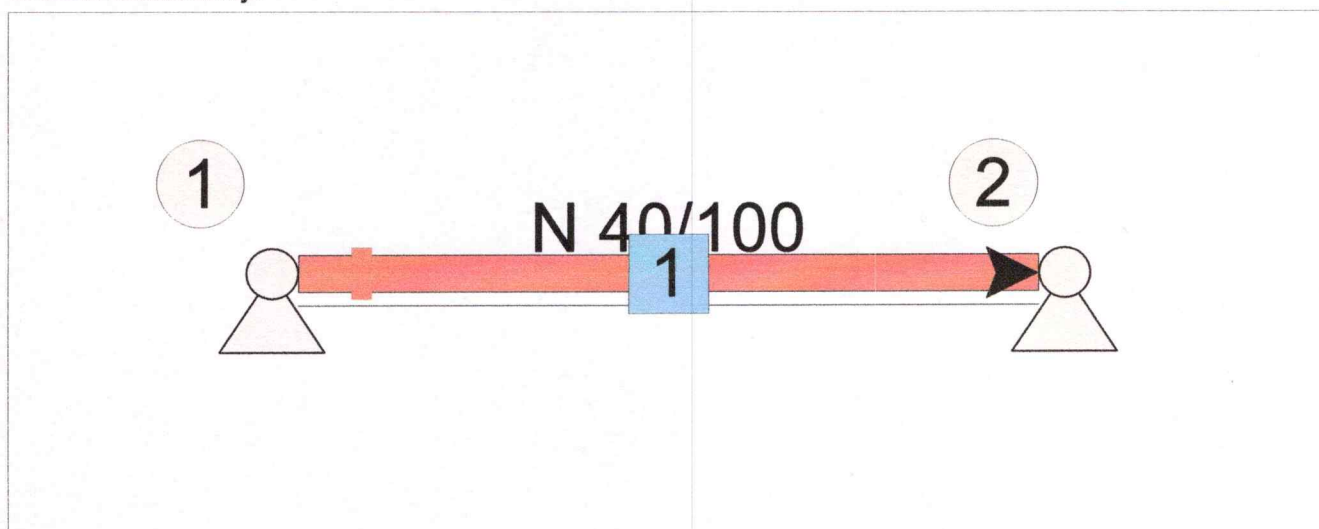


Tabela vozlišč

Index	X koor. [cm]	Z koor. [cm]	Podpore
1:	0.00000	0.00000	Ux Uz
2:	450.00	0.00000	Ux Uz

Tabela elementov

Index	Tip	Voz. 1	Voz. 2	Prečni prerez	Material	Dolžina [cm]	Spr. 1	Spr. 2
1:	nelin.	1:	2:	1: N 40/100	1: C 25/30	450.00	-	-

Tabela materialov

Index	Elastični modul [kN/cm ²]	Koef. termičnega raztezka [1/d°C]
1: C 25/30	3050.0	1.0000E-05
2: BSt500	20000.	1.2000E-05

Tabela prečnih prerezov

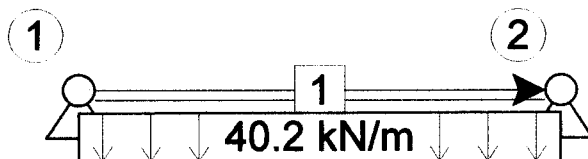
Index	A [cm ²]	I _y [cm ⁴]	Višina [cm]
1: N 40/100	4000.0	3.3333E+06	100.00

Obtežni vplivi in obtežni primeri

Obtežni primer	Mejno stanje	G	Q
G+Q	nosilnosti	1.35	1.50
G+Q/ser	uporabnosti	1.00	1.00

A1 : G

G + Lastna teža je avtomatično upoštevana.

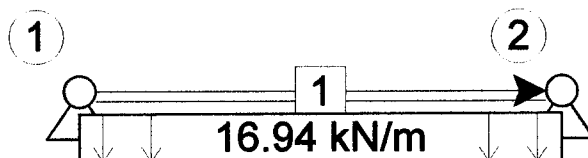


A1: G: Enakomerna zvezna obtežba elementov

Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Smer	Začetek	Konec	Lega	Ir
1:	0.00000	0.00000	40.200	40.200	0.00000	0.00000	Lokalni	0.00	1.00	Lokalni	C

A2 : Q

Q

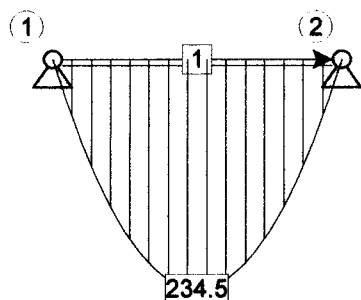


A2: Q: Enakomerna zvezna obtežba elementov

Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Smer	Začetek	Konec	Lega	Ir
1:	0.00000	0.00000	16.940	16.940	0.00000	0.00000	Lokalni	0.00	1.00	Lokalni	C

LC1: G+Q: Upogibni moment M_y

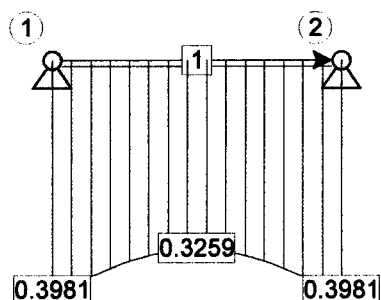
1.35 G + 1.50 Q



Enote: kNm

LC1: G+Q: Osna sila F_x

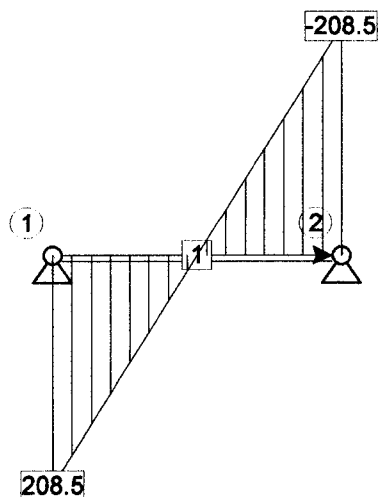
1.35 G + 1.50 Q



Enote: kN

LC1: G+Q: Prečna sila F_z

1.35 G + 1.50 Q



Enote: kN

LC1: G+Q: Vozlišni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
1:	-	-	-0.019827
2:	-	-	0.019827

LC1: G+Q: Reakcije

Voz.	Fx [kN]	Fz [kN]	My [kNm]
1:	-0.32594	-208.48	-
2:	0.32594	-208.48	-

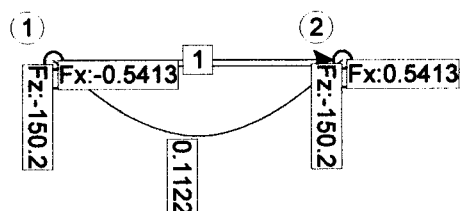
LC1: G+Q: Element 1:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.39808	208.48	0.00000
0.05	0.38993	187.63	44.563
0.10	0.38042	166.79	84.435
0.15	0.37030	145.94	119.62
0.20	0.36022	125.09	150.11
0.25	0.35074	104.24	175.91
0.30	0.34233	83.393	197.02
0.35	0.33538	62.545	213.43
0.40	0.33021	41.696	225.16
0.45	0.32702	20.848	232.20
0.50	0.32594	0.00000	234.54

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.32702	-20.848	232.20
0.60	0.33021	-41.696	225.16
0.65	0.33538	-62.545	213.43
0.70	0.34233	-83.393	197.02
0.75	0.35074	-104.24	175.91
0.80	0.36022	-125.09	150.11
0.85	0.37030	-145.94	119.62
0.90	0.38042	-166.79	84.435
0.95	0.38993	-187.63	44.563
1.00	0.39808	-208.48	0.00000

LC2: G+Q/ser: Pomiki in reakcije

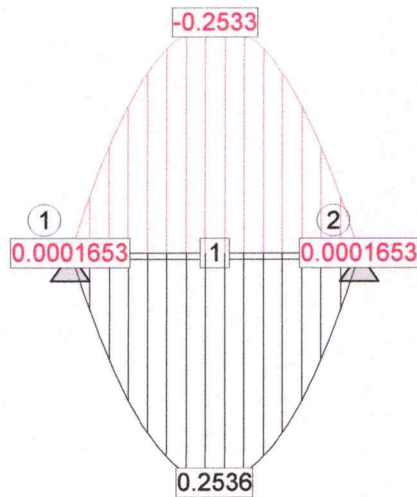
1.00 G + 1.00 Q



Enote: cm, kN, kNm

LC2: G+Q/ser: Napetost

1.00 G + 1.00 Q



Enote: kN/cm²

LC2: G+Q/ser: Vozlišni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
1:	-	-	-0.045708
2:	-	-	0.045708

LC2: G+Q/ser: Reakcije

Voz.	Fx [kN]	Fz [kN]	My [kNm]
1:	-0.54133	-150.20	-
2:	0.54133	-150.20	-

LC2: G+Q/ser: Element 1:

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.00	0.66115	150.20	0.00000
0.05	0.64760	135.18	32.104
0.10	0.63182	120.16	60.829
0.15	0.61501	105.14	86.175
0.20	0.59827	90.117	108.14
0.25	0.58252	75.098	126.73
0.30	0.56855	60.078	141.93
0.35	0.55702	45.059	153.76
0.40	0.54842	30.039	162.21
0.45	0.54312	15.020	167.28
0.50	0.54133	0.00000	168.97

Lega	Fx [kN]	Fz [kN]	My [kNm]
0.55	0.54312	-15.020	167.28
0.60	0.54842	-30.039	162.21
0.65	0.55702	-45.059	153.76
0.70	0.56855	-60.078	141.93
0.75	0.58252	-75.098	126.73
0.80	0.59827	-90.117	108.14
0.85	0.61501	-105.14	86.175
0.90	0.63182	-120.16	60.829
0.95	0.64760	-135.18	32.104
1.00	0.66115	-150.20	0.00000

Rezultati za element 1 (Preverjeno po EC2):

Prečni prerez [cm]

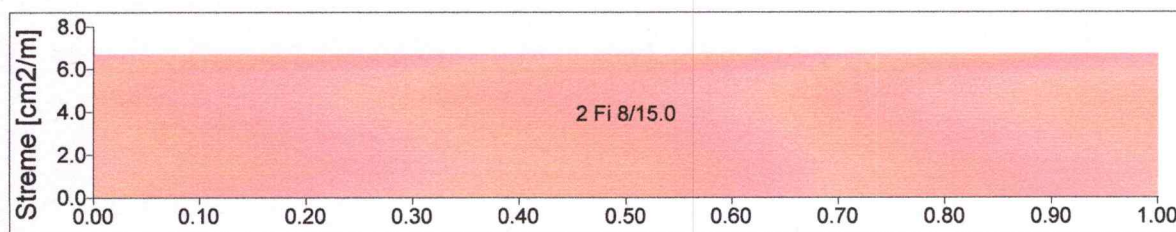
[N 40/100]
 $h = 100.00$
 $b = 40.00$

Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Beton: C 25/30
 Armatura: BSt500

Stremenska armatura

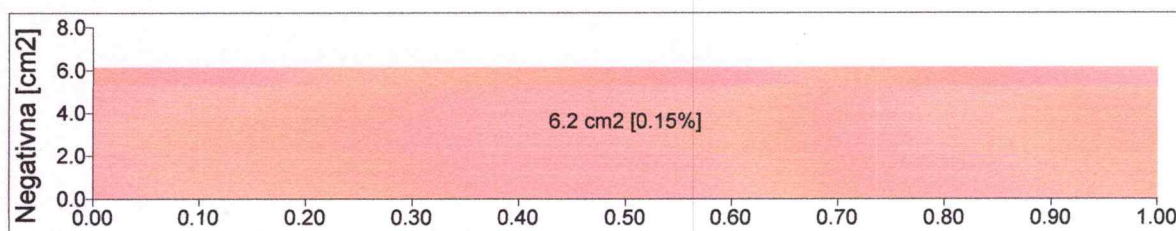
n	Fi [mm]	e [cm]	xs	xe
2	8.0000	15.000	0.00	1.00



Negativna armatura

Vrstica	n	Fi [mm]	xs	xe
1	4	14.000	0.00	1.00

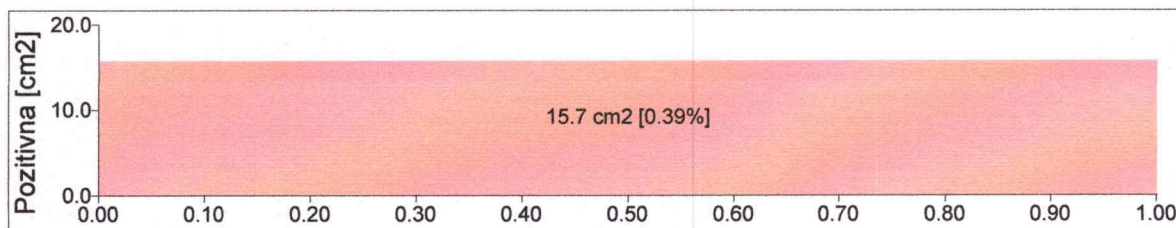
Zaščitni sloj znaša 3.0000 cm.



Pozitivna armatura

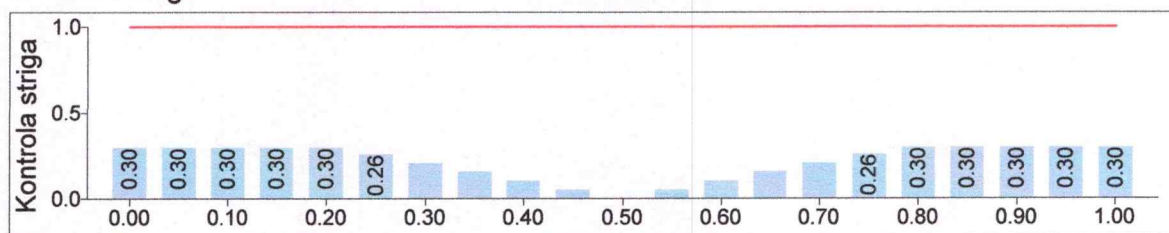
Vrstica	n	Fi [mm]	xs	xe
1	5	20.000	0.00	1.00

Zaščitni sloj znaša 3.0000 cm.

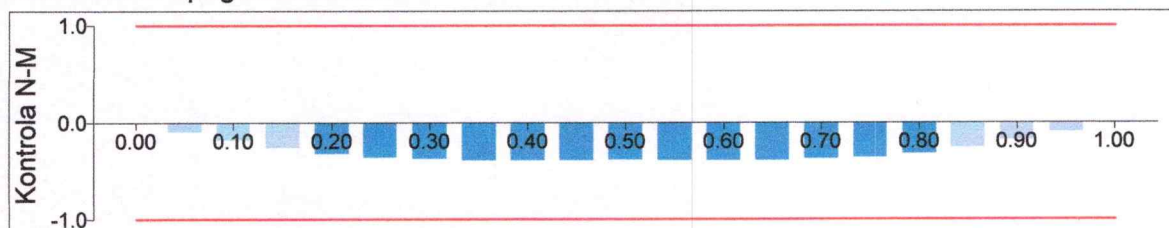


Mejno stanje nosilnosti

Kontrola striga

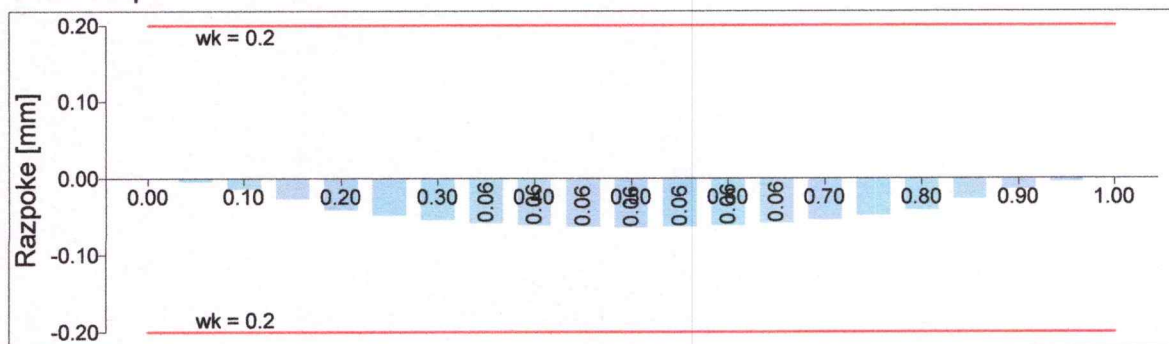


Oсна sila in upogibni moment



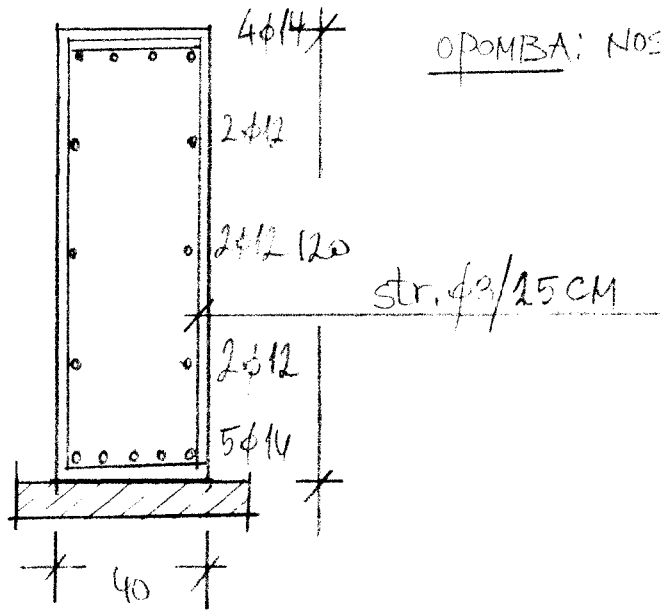
Mejno stanje uporabnosti

Širina razpok



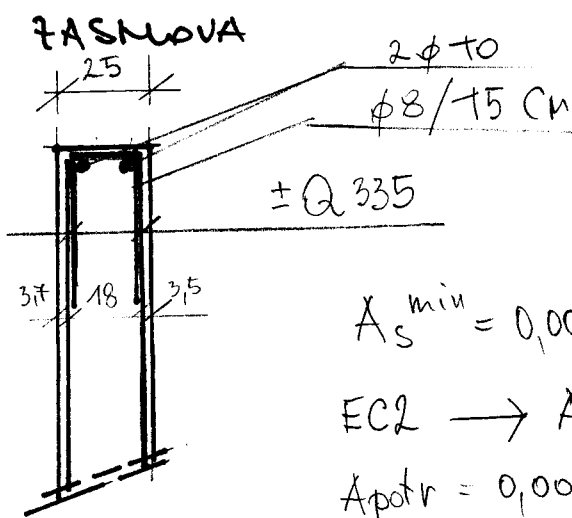
POZ (7) - A.B. NOSILEC $B/H = 40/120 \text{ cm}$; BSt 500; C25/30

ŽASNOVA



OPOMBA: NOSILEC ARMIRANO KONSTRUKTIVNI

POZ (8) - oporni žig $D = 25,0 \text{ cm}$; BSt 500; C25/30, XC2



Minimalna uporišna armatura glede na zahteve EC

EC2 $\rightarrow 0,4\%$ skupni presek stene
EC8 $\rightarrow 0,2\%$ srednji del stene in 1% robovi stene

$$A_s^{\min} = 0,0013 \times 25 \times 100 = 3,25 \text{ cm}^2 (\pm Q 335)$$

$$\text{EC2} \rightarrow A_s = 20 \times 100 = 2000 \text{ cm}^2$$

$$A_{\text{potr}} = 0,004 \times 2000 = 8,0 \text{ cm}^2$$

OBOJESTRANSKA ARMATURA!

SREDNJI DEL STENE

$$\text{EC8} \rightarrow A_{\text{SR.DEL}} = 0,002 \times 100 \times 25 = 5,0 \text{ cm}^2$$

OBOJESTRANSKA ARMATURA

POZ 9-OPORNI ZID IN TRIBUNE D=25 CM; BSt500; C25/30

Shema konstrukcije

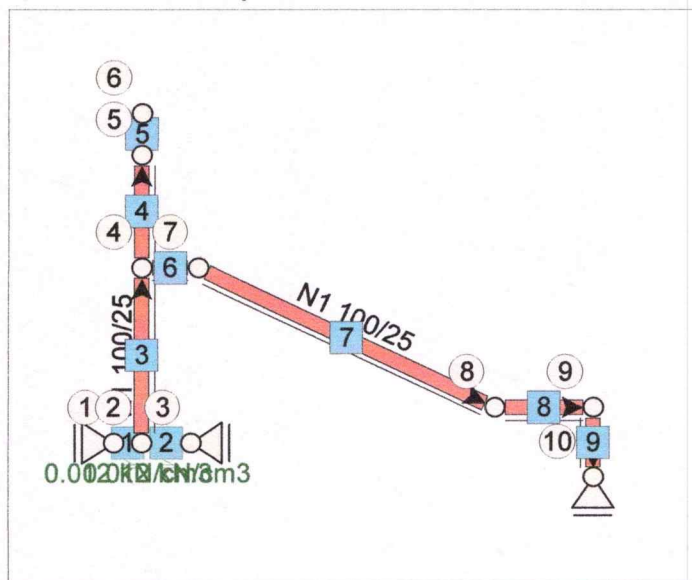


Tabela vozlišč

Index	X koor. [cm]	Z koor. [cm]	Podpore
1:	0.00000	-50.000	Ux
2:	40.000	-50.000	-
3:	110.00	-50.000	Ux
4:	40.000	-300.00	-
5:	40.000	-460.00	-

Index	X koor. [cm]	Z koor. [cm]	Podpore
6:	40.000	-520.00	-
7:	120.00	-300.00	-
8:	540.00	-100.00	-
9:	680.00	-100.00	-
10:	680.00	0.00000	Uz

Tabela elementov

Index	Tip	Voz. 1	Voz. 2	Prečni prerez	Material	Dolžina [cm]	Spr. 1	Spr. 2
1:	nelin.	1:	2:	2: T 100/100	1: C 25/30	40.000	-	-
2:	nelin.	2:	3:	2: T 100/100	1: C 25/30	70.000	-	-
3:	nelin.	2:	4:	1: N1 100/25	1: C 25/30	250.00	-	-
4:	nelin.	4:	5:	1: N1 100/25	1: C 25/30	160.00	-	-
5:	nelin.	5:	6:	1: N1 100/25	1: C 25/30	60.000	-	-
6:	nelin.	4:	7:	1: N1 100/25	1: C 25/30	80.000	-	-
7:	nelin.	7:	8:	1: N1 100/25	1: C 25/30	465.19	-	-
8:	nelin.	8:	9:	1: N1 100/25	1: C 25/30	140.00	-	-
9:	nelin.	9:	10:	1: N1 100/25	1: C 25/30	100.00	-	-

Element na elastični podlagi

Index	Lega	Modul reakcije tal [kN/cm ³]
1:	Pozitivna stran	0.012000
2:	Pozitivna stran	0.012000

Tabela materialov

Index	Elastični modul [kN/cm ²]	Koef. termičnega raztezka [1/d°C]
1: C 25/30	3050.0	1.0000E-05
2: BSt500	20000.	1.2000E-05

Tabela prečnih prerezov

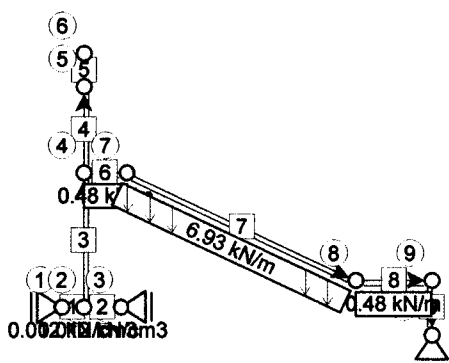
Index	A [cm ²]	I _y [cm ⁴]	Višina [cm]
1: N1 100/25	2500.0	1.3021E+05	25.000
2: T 100/100	10000.	8.3333E+06	100.00

Obtežni vplivi in obtežni primeri

Obtežni primer	Mejno stanje	G	Q
G+Q	nosilnosti	1.35	1.50
G+Q/ser	uporabnosti	1.00	1.00

A1 : G

G + Lastna teža je avtomatično upoštevana.



A1: G: Enakomerna zvezna obtežba elementov

Element	F _{x s} [kN/m]	F _{x e} [kN/m]	F _{z s} [kN/m]	F _{z e} [kN/m]	M _{y s} [kNm/m]	M _{y e} [kNm/m]	Smer	Začetek	Konec	Lega
6:	0.00000	0.00000	0.48000	0.48000	0.00000	0.00000	Lokalni	0.00	1.00	Lokaln
7:	0.00000	0.00000	6.9300	6.9300	0.00000	0.00000	Globalni	0.00	1.00	Lokaln
8:	0.00000	0.00000	0.48000	0.48000	0.00000	0.00000	Globalni	0.00	1.00	Lokaln

The diagram shows a frame structure with the following components:

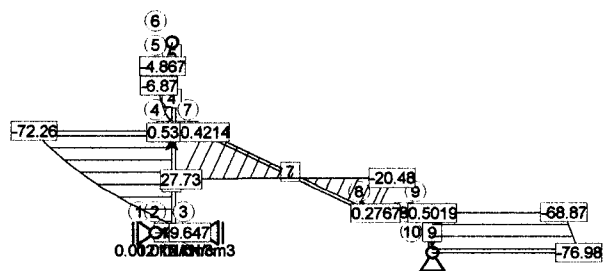
- Vertical Member:**
 - Support at the bottom: Pin support (1) and roller support (2).
 - Applied loads: A horizontal point load of 17.5 kN (3) and a horizontal point load of 17.5 kN/m (4) acting to the right. A vertical point load of 17.5 kN (5) acts downwards at the top.
 - Dimensions: 0.00 m (6) and 1.2 m (7) are indicated along the vertical member.
- Diagonal Member:**
 - Support at the bottom: Pin support (8) and roller support (9).
 - Applied loads: A horizontal point load of 4 kN (10) and a horizontal point load of 4 kN/m (11) acting to the right. A vertical point load of 4.38 kN/m (12) acts downwards.
 - Dimensions: 0.00 m (13) and 1.2 m (14) are indicated along the diagonal member.

Element	Fx s [kN/m]	Fx e [kN/m]	Fz s [kN/m]	Fz e [kN/m]	My s [kNm/m]	My e [kNm/m]	Smer	Začetek	Konec	Lega
3:	37.000	17.500	0.00000	0.00000	0.00000	0.00000	Lokalni	0.00	1.00	Lokaln
4:	17.500	0.00000	0.00000	0.00000	0.00000	0.00000	Lokalni	0.00	1.00	Lokaln
6:	0.00000	0.00000	4.0000	4.0000	0.00000	0.00000	Globalni	0.00	1.00	Lokaln
7:	0.00000	0.00000	4.3800	4.3800	0.00000	0.00000	Globalni	0.00	1.00	Lokaln
8:	0.00000	0.00000	4.0000	4.0000	0.00000	0.00000	Globalni	0.00	1.00	Lokaln

[illegible]

LC1: G+Q: Osnova sila Fx

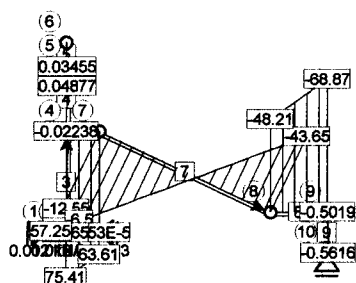
1.35 G + 1.50 Q



Enote: kN

LC1: G+Q: Prečna sila Fz

1.35 G + 1.50 Q



Enote: kN

LC1: G+Q: Vozlišni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
1:	-	-0.20032	-0.38602
2:	0.00000	0.069172	-0.38603
3:	-	0.54078	-0.38601
4:	1.7301	0.076386	-0.40660
5:	2.8657	0.080507	-0.40670

Voz.	ux [cm]	uz [cm]	fiy [deg]
6:	3.2916	0.082038	-0.40670
7:	1.7282	0.63272	-0.37960
8:	1.5738	0.94920	0.32884
9:	1.5706	0.0036152	0.41755
10:	2.2998	-	0.41794

LC1: G+Q: Reakcije

Voz.	Fx [kN]	Fz [kN]	My [kNm]
1:	0.00000	-	-
3:	0.00000	-	-

Voz.	Fx [kN]	Fz [kN]	My [kNm]
10:	-	-76.985	-

LC2: G+Q/ser: Vozlišni pomiki

Voz.	ux [cm]	uz [cm]	fiy [deg]
1:	-	-0.73795	-0.80301
2:	0.00000	-0.17734	-0.80303
3:	-	0.80359	-0.80285
4:	3.7525	-0.14580	-0.91469
5:	6.3076	-0.12514	-0.91524

Voz.	ux [cm]	uz [cm]	fiy [deg]
6:	7.2660	-0.11744	-0.91525
7:	3.7425	1.1175	-0.86984
8:	3.2730	2.0623	0.70556
9:	3.2580	0.014764	0.90830
10:	4.8454	-	0.91023

LC2: G+Q/ser: Reakcije

Voz.	Fx [kN]	Fz [kN]	My [kNm]
1:	0.00000	-	-
3:	0.00000	-	-

Voz.	Fx [kN]	Fz [kN]	My [kNm]
10:	-	-54.551	-

LC2: G+Q/ser: Element 1:

Lega	Fx [kN]	Fz [kN]	My [kNm]	Tlak [kPa]
0.00	0.00000	0.00000	0.00000	0.00000
0.05	0.00000	-0.48069	-0.0048069	0.00000
0.10	0.00000	-0.96138	-0.019228	0.00000
0.15	0.00000	-1.4421	-0.043262	0.00000
0.20	0.00000	-1.9228	-0.076910	0.00000
0.25	0.00000	-2.4035	-0.12017	0.00000
0.30	0.00000	-2.8841	-0.17305	0.00000
0.35	0.00000	-3.3648	-0.23554	0.00000
0.40	0.00000	-3.8455	-0.30764	0.00000
0.45	0.00000	-4.3262	-0.38936	0.00000
0.50	0.00000	-4.8069	-0.48069	0.00000
0.55	0.00000	-5.2876	-0.58163	0.00000
0.60	0.00000	-5.7683	-0.69219	0.00000
0.65	0.00000	-6.2490	-0.81237	0.00000
0.70	0.00000	-6.7297	-0.94215	0.00000
0.75	0.00000	-7.2104	-1.0816	0.00000
0.80	0.00000	-7.6910	-1.2306	0.00000
0.85	0.00000	-8.1717	-1.3892	0.00000
0.90	0.00000	-8.6524	-1.5574	0.00000
0.95	0.00000	-9.1331	-1.7353	0.00000
1.00	0.00000	-9.6138	-1.9228	0.00000

LC2: G+Q/ser: Pomiki in reakcije

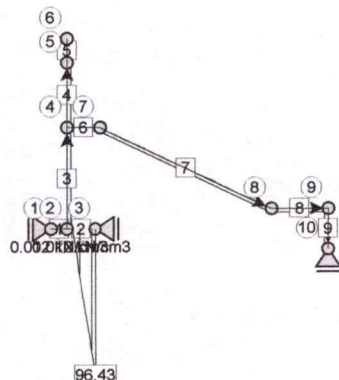
1.00 G + 1.00 Q



Enote: cm, kN, kNm

LC2: G+Q/ser: Kontaktni tlak

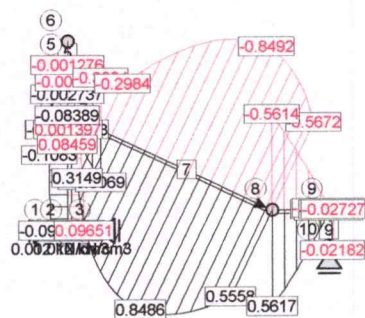
1.00 G + 1.00 Q



Enote: kPa

LC2: G+Q/ser: Napetost

1.00 G + 1.00 Q

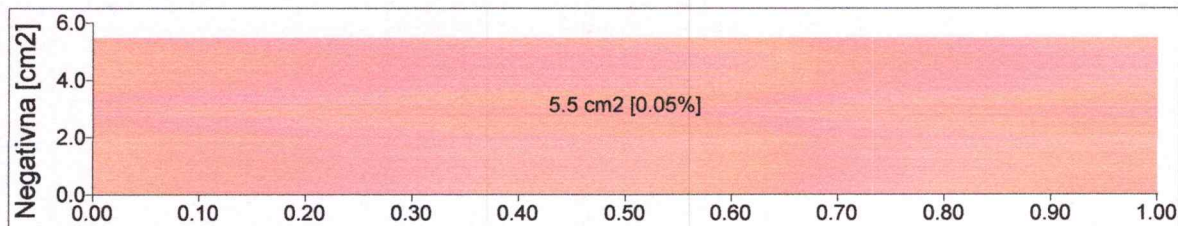


Enote: kN/cm²

Negativna armatura

Vrstica	n	Fi [mm]	xs	xe
1	7	10.000	0.00	1.00

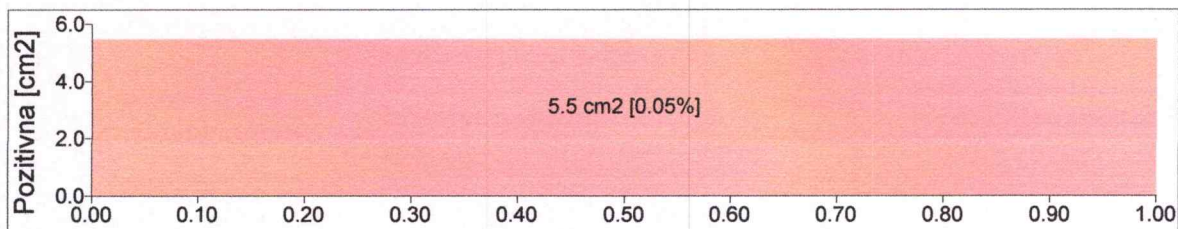
Zaščitni sloj znaša 3.0000 cm.



Pozitivna armatura

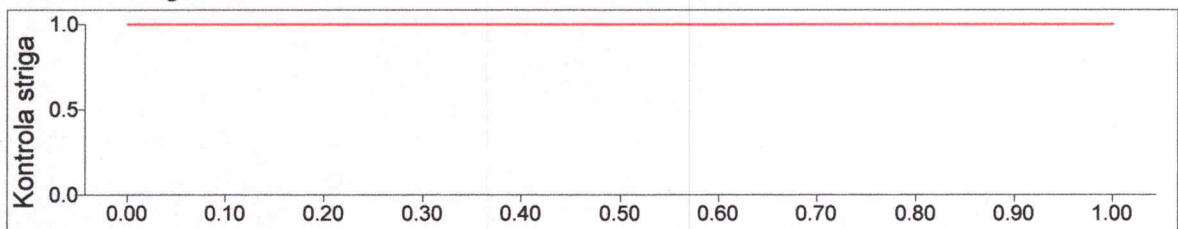
Vrstica	n	Fi [mm]	xs	xe
1	7	10.000	0.00	1.00

Zaščitni sloj znaša 3.0000 cm.

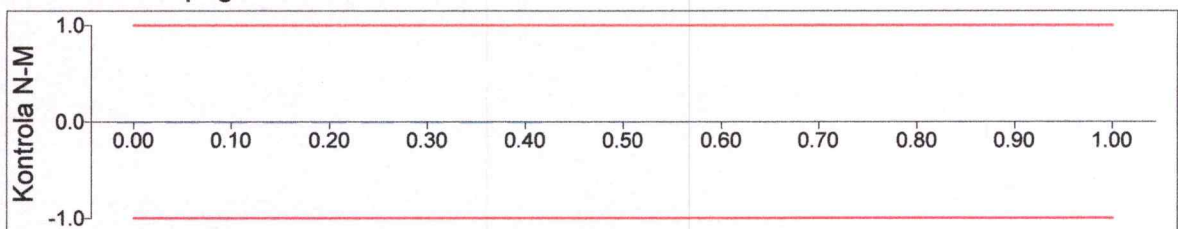


Mejno stanje nosilnosti

Kontrola striga

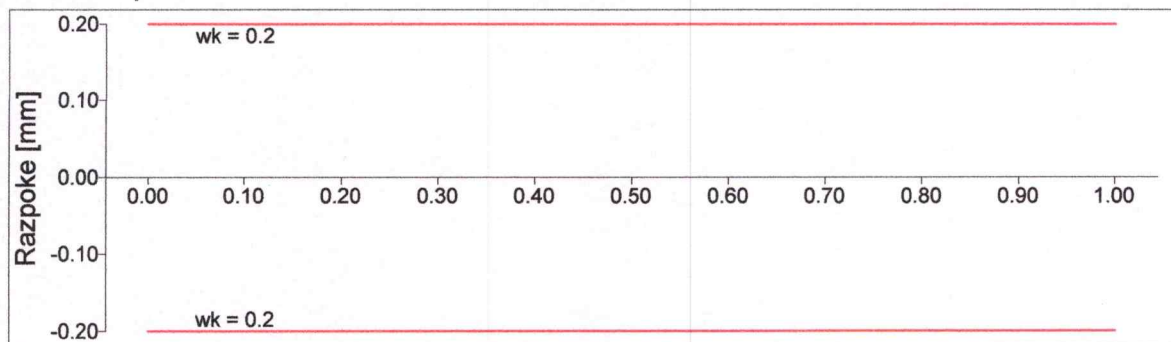


Osna sila in upogibni moment



Mejno stanje uporabnosti

Širina razpok



Rezultati za element 6 (Preverjeno po EC2):

Prečni prerez [cm]

[N1 100/25]

$h = 25.00$

$b = 100.00$

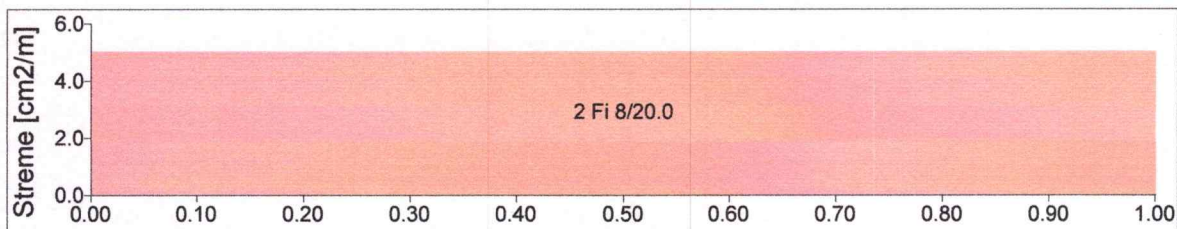
Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Beton: C 25/30

Armatura: BSt500

Stremenska armatura

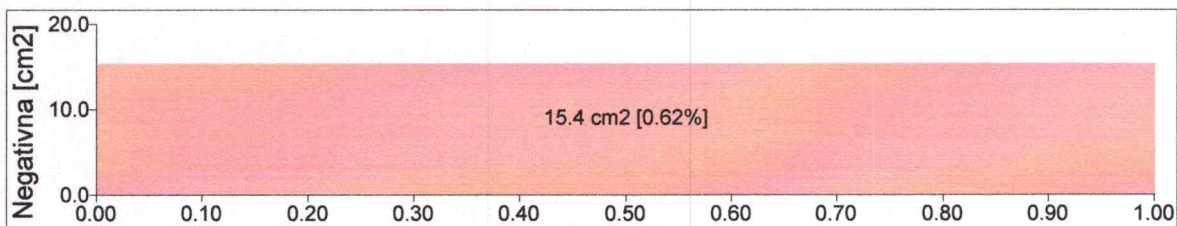
n	Fi [mm]	e [cm]	xs	xe
2	8.0000	20.000	0.00	1.00



Negativna armatura

Vrstica	n	Fi [mm]	xs	xe
1	10	14.000	0.00	1.00

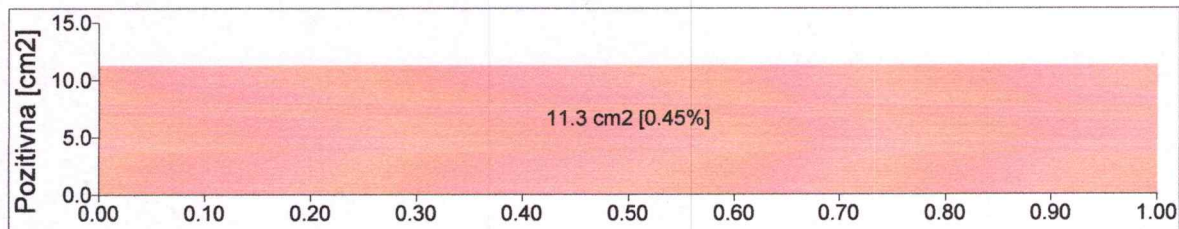
Zaščitni sloj znaša 3.0000 cm.



Pozitivna armatura

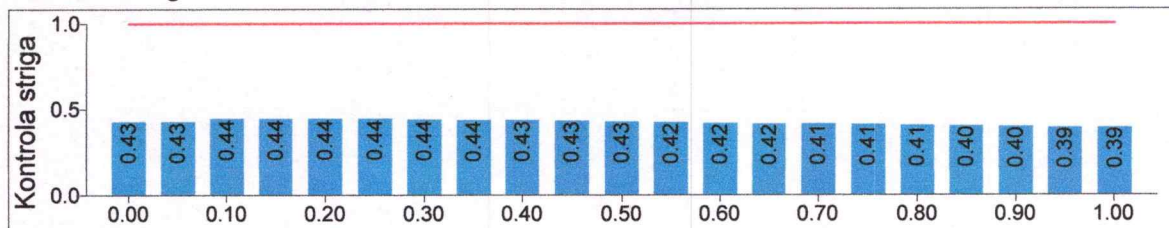
Vrstica	n	Fi [mm]	xs	xe
1	10	12.000	0.00	1.00

Zaščitni sloj znaša 3.0000 cm.

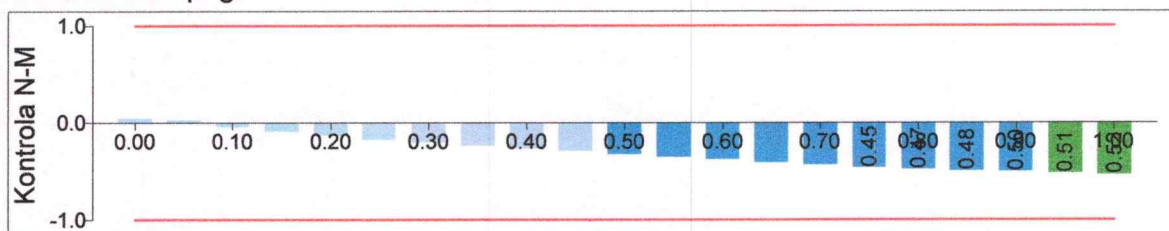


Mejno stanje nosilnosti

Kontrola striga

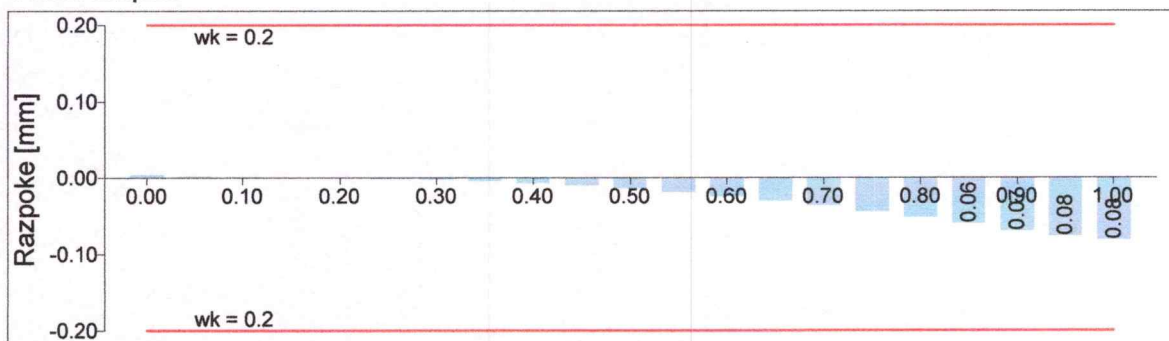


Oсна sila in upogibni moment



Mejno stanje uporabnosti

Širina razpok



Rezultati za element 7 (Preverjeno po EC2):

Prečni prerez [cm]

[N1 100/25]

h = 25.00

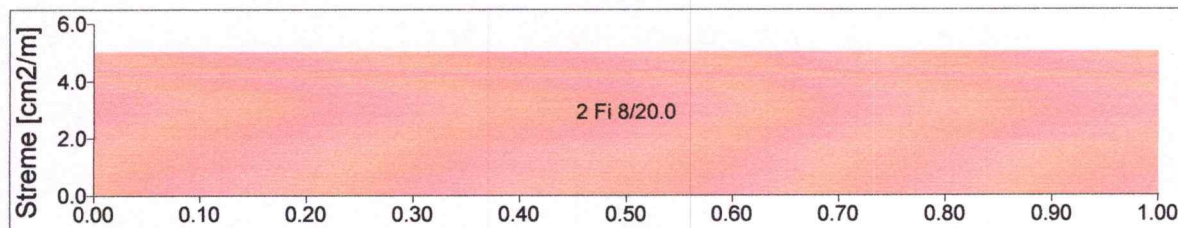
b = 100.00

Širini podpor elementa levo = 0.00 cm, desno = 0.00 cm

Beton: C 25/30
Armatura: BSt500

Stremenska armatura

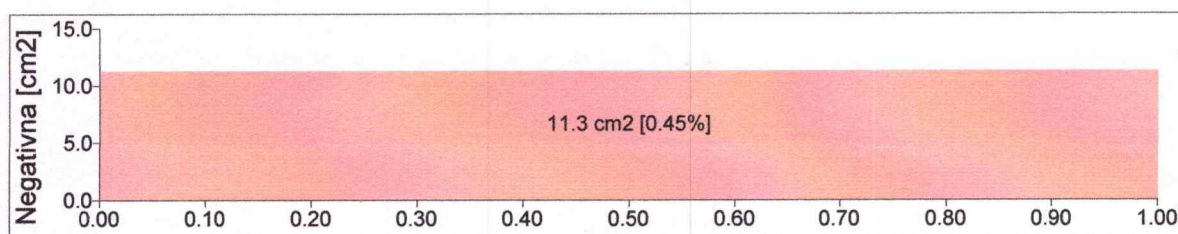
n	Fi [mm]	e [cm]	xs	xe
2	8.0000	20.000	0.00	1.00



Negativna armatura

Vrstica	n	Fi [mm]	xs	xe
1	10	12.000	0.00	1.00

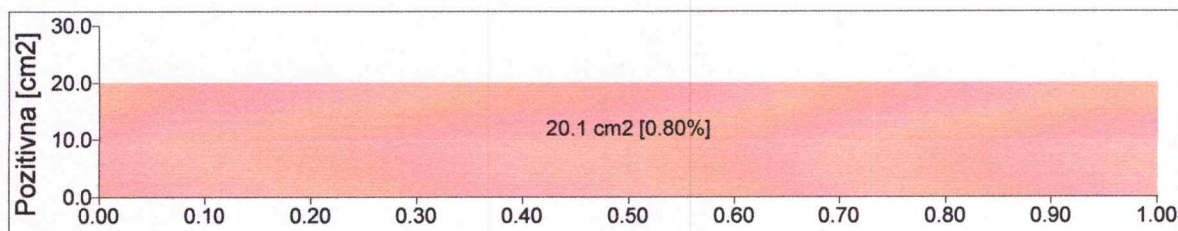
Zaščitni sloj znaša 3.0000 cm.



Pozitivna armatura

Vrstica	n	Fi [mm]	xs	xe
1	10	16.000	0.00	1.00

Zaščitni sloj znaša 3.0000 cm.



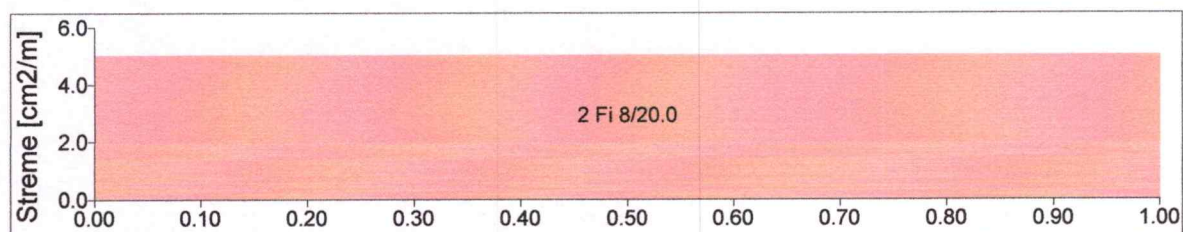
Mejno stanje nosilnosti

Kontrolna striga	Frequency
0.00	0.31
0.10	0.30
0.20	0.27
0.30	0.20
0.40	0.15
0.50	0.10
0.60	0.05
0.70	0.05
0.80	0.10
0.90	0.15
1.00	0.20

Sample	Kontrola N-M
1	0.00
2	0.43
3	0.50
4	0.56
5	0.62
6	0.66
7	0.70
8	0.73
9	0.75
10	0.77
11	0.78
12	0.78
13	0.77
14	0.76
15	0.74
16	0.71
17	0.67
18	0.63
19	0.58
20	0.50

wk	Razpoke [mm]
0.00	0.00
0.05	-0.05
0.10	-0.06
0.15	-0.07
0.20	-0.08
0.25	-0.09
0.30	-0.10
0.35	-0.10
0.40	-0.11
0.45	-0.11
0.50	-0.11
0.55	-0.11
0.60	-0.11
0.65	-0.11
0.70	-0.11
0.75	-0.11
0.80	-0.10
0.85	-0.09
0.90	-0.08
0.95	-0.07
1.00	-0.06

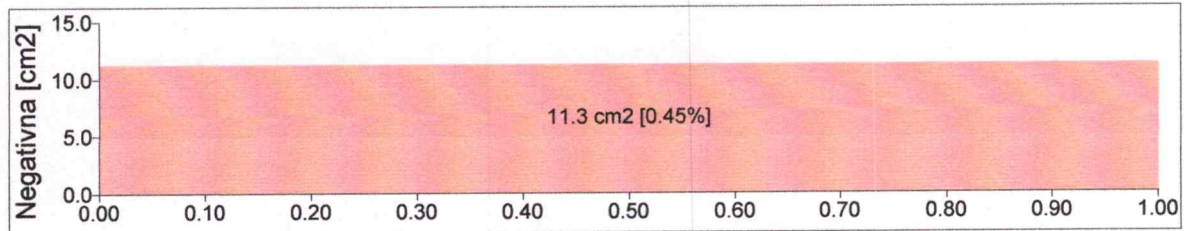
n	Fi [mm]	e [cm]	xs	xe
2	8.0000	20.000	0.00	1.00



Negativna armatura

Vrstica	n	Fi [mm]	xs	xe
1	10	12.000	0.00	1.00

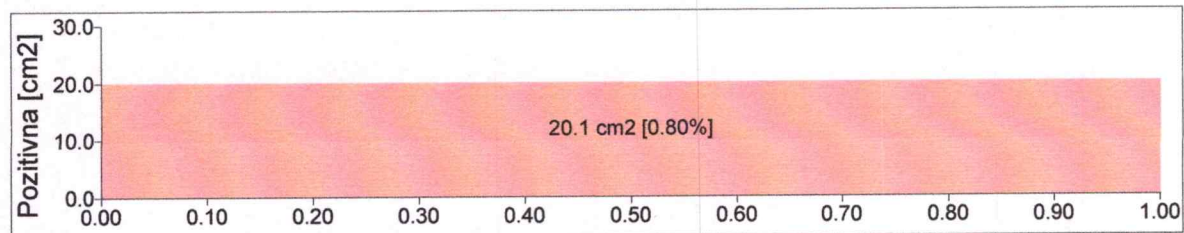
Zaščitni sloj znaša 3.0000 cm.



Pozitivna armatura

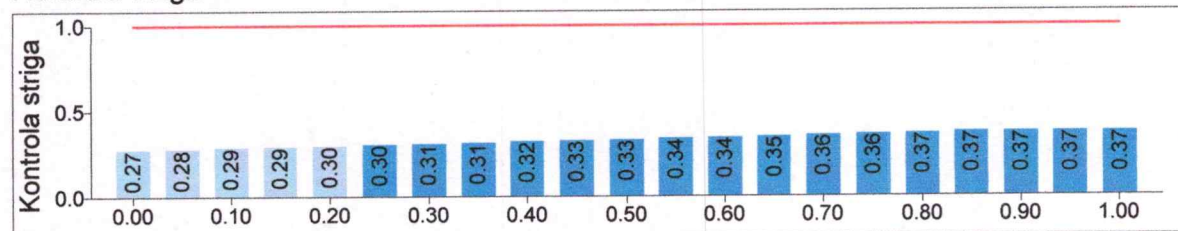
Vrstica	n	Fi [mm]	xs	xe
1	10	16.000	0.00	1.00

Zaščitni sloj znaša 3.0000 cm.

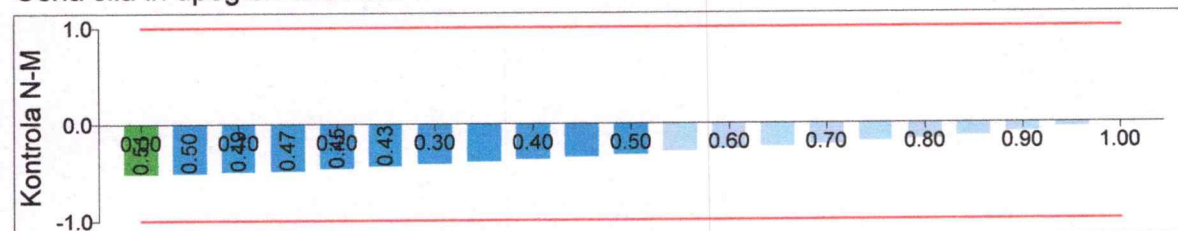


Mejno stanje nosilnosti

Kontrola striga



Osna sila in upogibni moment



Mejno stanje uporabnosti

medosni razdalji 4,80 m. Jekleni okvirji bodo na isti medosni razdalji. Povezava med točkovnimi temelji je izvedena z dvema AB nosilcema (POZ 6 in POZ 7, dimenzij 40/100 cm in 40/120 cm). Temeljenje tribun se bo izvedlo na eni strani z pasovnim temeljem dimenzij 60x120 cm na drugi strani na A.B. nosilec (POZ 6) in temeljni steber POZ 4).

Pasovni in točkovni temelji se morajo izvesti na komprimiranem nasutju ($MV = 80 \text{ MPa}$) in podložnem betonu kvalitete C8/10 debeline najmanj 8,0 cm.

MATERIALI

- Beton (vse konstrukcije, ki so v stiku z zemljino): C25/30 XC2
- Beton (vse konstrukcije, ki niso v stiku z zemljino): C25/30 XC1
- Beton podložni, ki so v stiku z zemljino: C8/10 X0
- Armaturno železo B 500 B
- Jekleni elementi:
- Izračunani elementi S 235 JO, S355 JO;
- Izvedbeni razred jeklene konstrukcije **EXC2**:
- Razred glede posledic –CC2
- Glede na projektiranje-SC1
- Glede na izvedbo-PC1

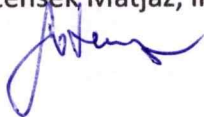
GEOMETRIJA IN OBTEŽBE

Geometrija in obtežbe so razvidne iz statičnega izračuna ter armaturnih načrtov, ki sledijo v nadaljevanju.

REZULTATI

Izračun pokaže, da je predpostavljena konstrukcija ustrezna.

Sestavil: Sotenšek, Matjaž, inž.gr.

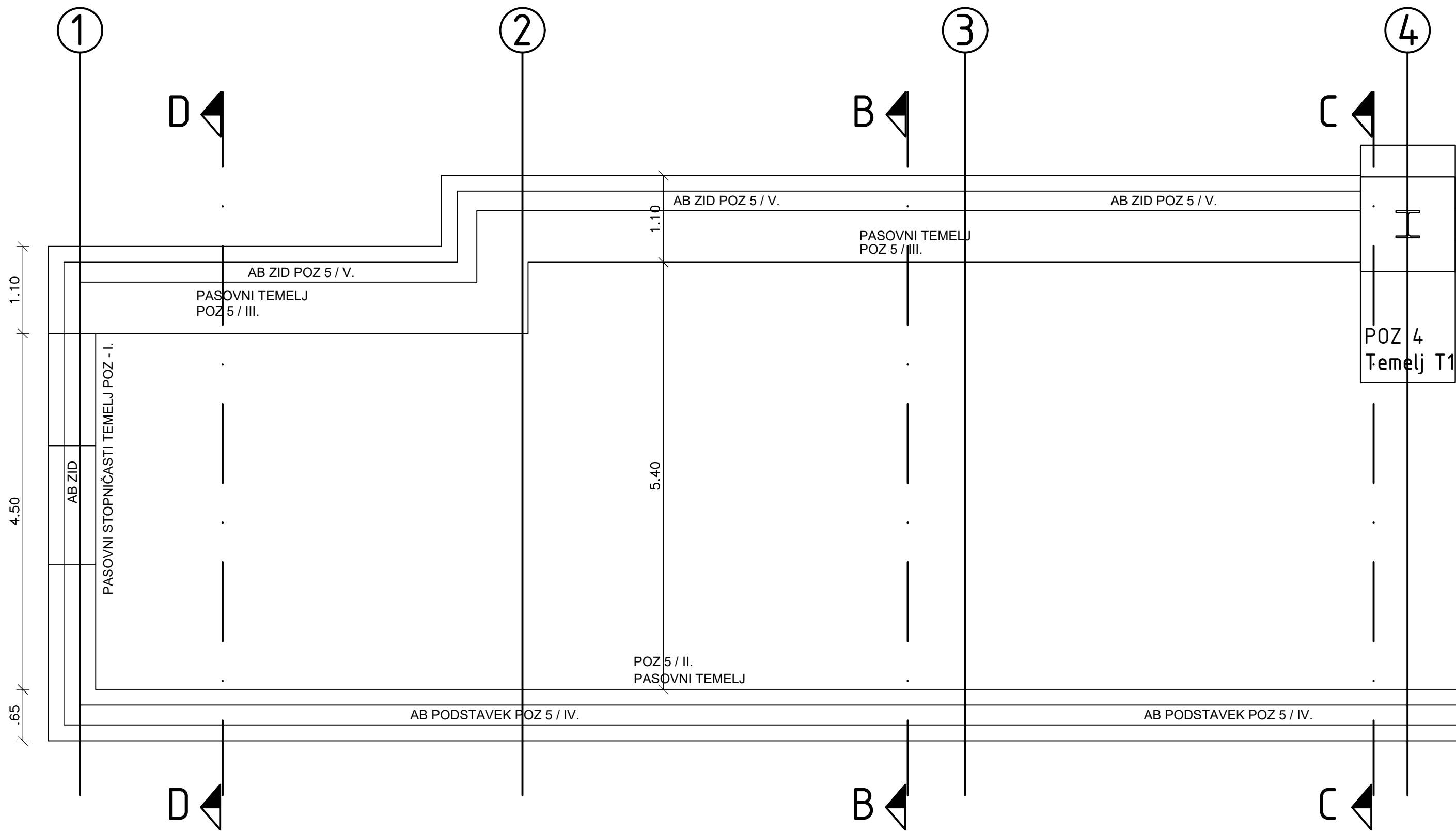


Avgust 2026

SPECIFIKACIJA ARMATURE ZA : TRIBUNE LITIJA**Investitor:**

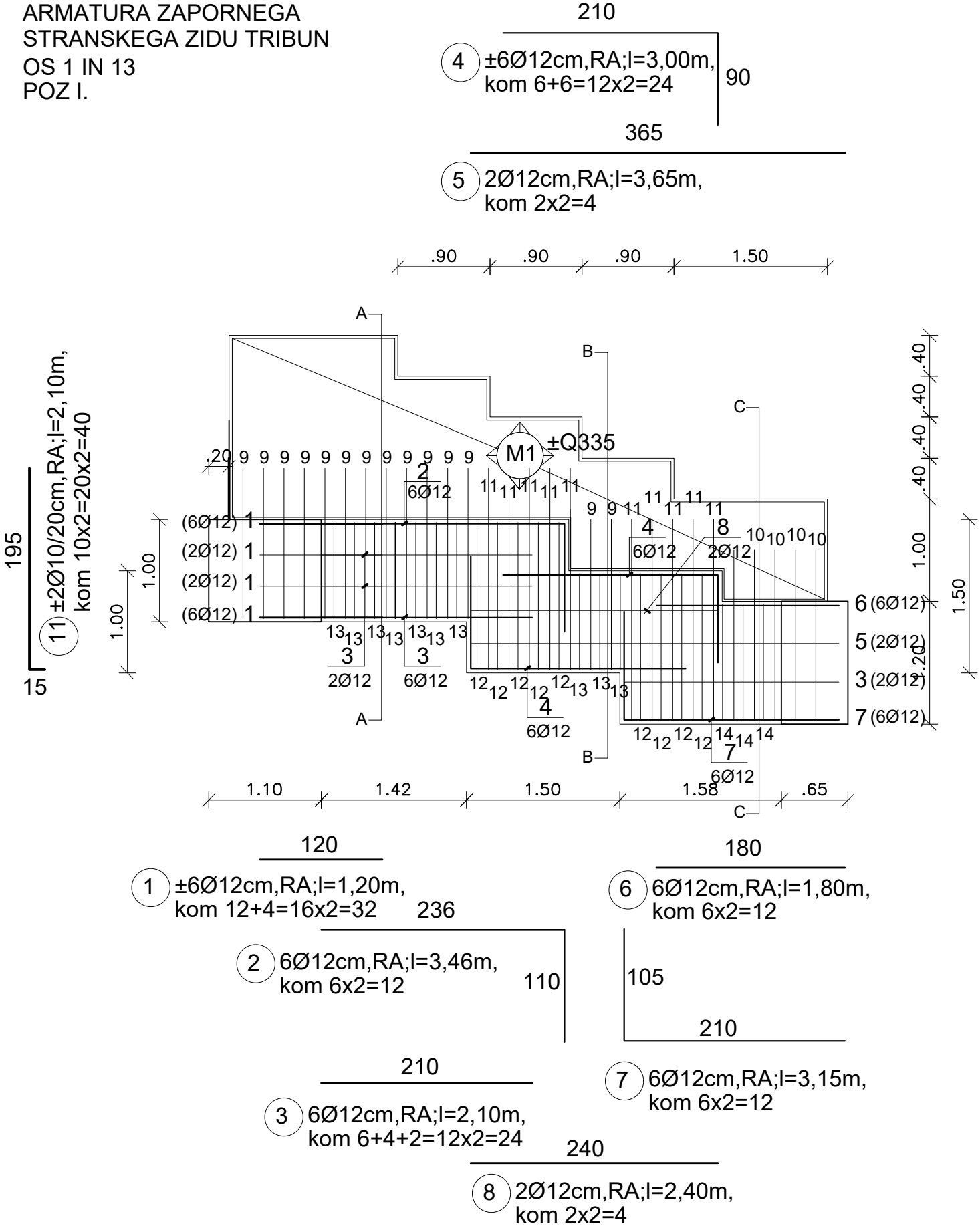
LIST ŠT.	DO FI 12	NAD FI 12	R MREŽE	Q MREŽE
2-3	451,08			
4-8	3346,06			
9-10	397,7			2229,49
12-15	9506,94	3313,82		
18-19	3464,04			
20-23	3410,32	418,62		
25-26	214,67	834,61		
27-30	5044,27	4016,16		
31	125,08			947,33
SKUPAJ:	25960,16	8583,21	0	3176,82

37720,19

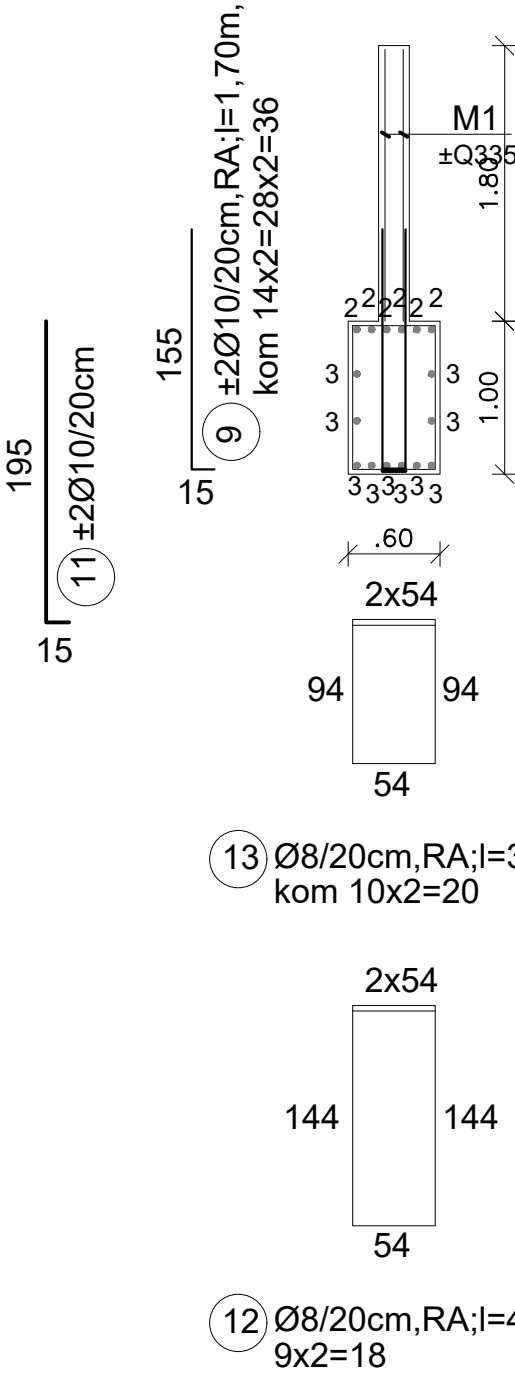


POZICIJE - TEMELJI - TRIBUNE					
OS 1- 4 IN OS 10-13					
Gradbene konstrukcije					
MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT.	1

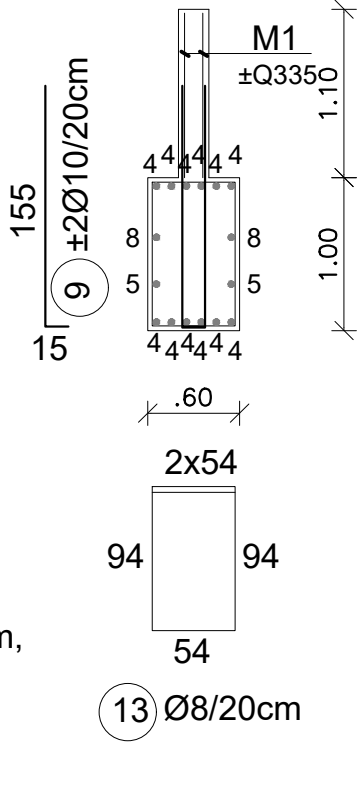
ARMATURA ZAPORNEGA
STRANSKEGA ZIDU TRIBUN
OS 1 IN 13
POZ I.



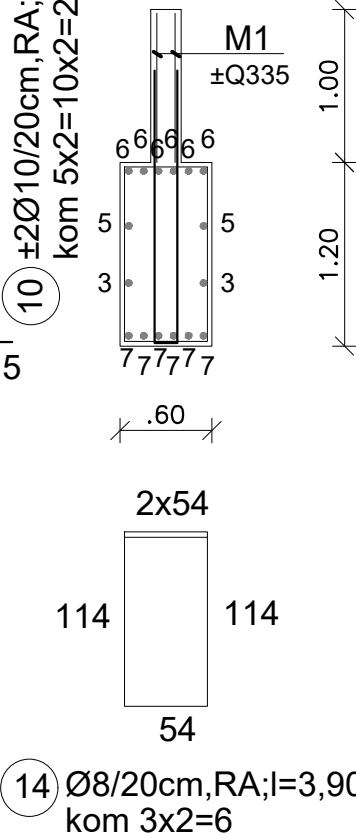
PREREZ A-A



PREREZ B-B



PREREZ C-C



SPECIFIKACIJA NA NASLEDNJEM LISTU ŠT. 3

ARMATURA STOP. PASOVNEGA TEMELJA IN
ZAPORNEGA STRANSKEGA ZIDU TRIBUN POZ I.

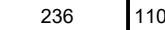
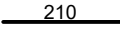
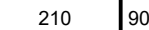
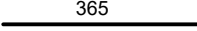
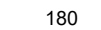
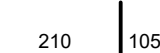
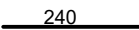
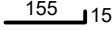
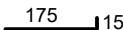
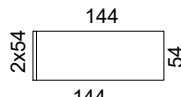
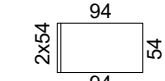
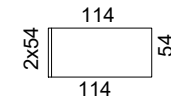
OS 1 IN 13

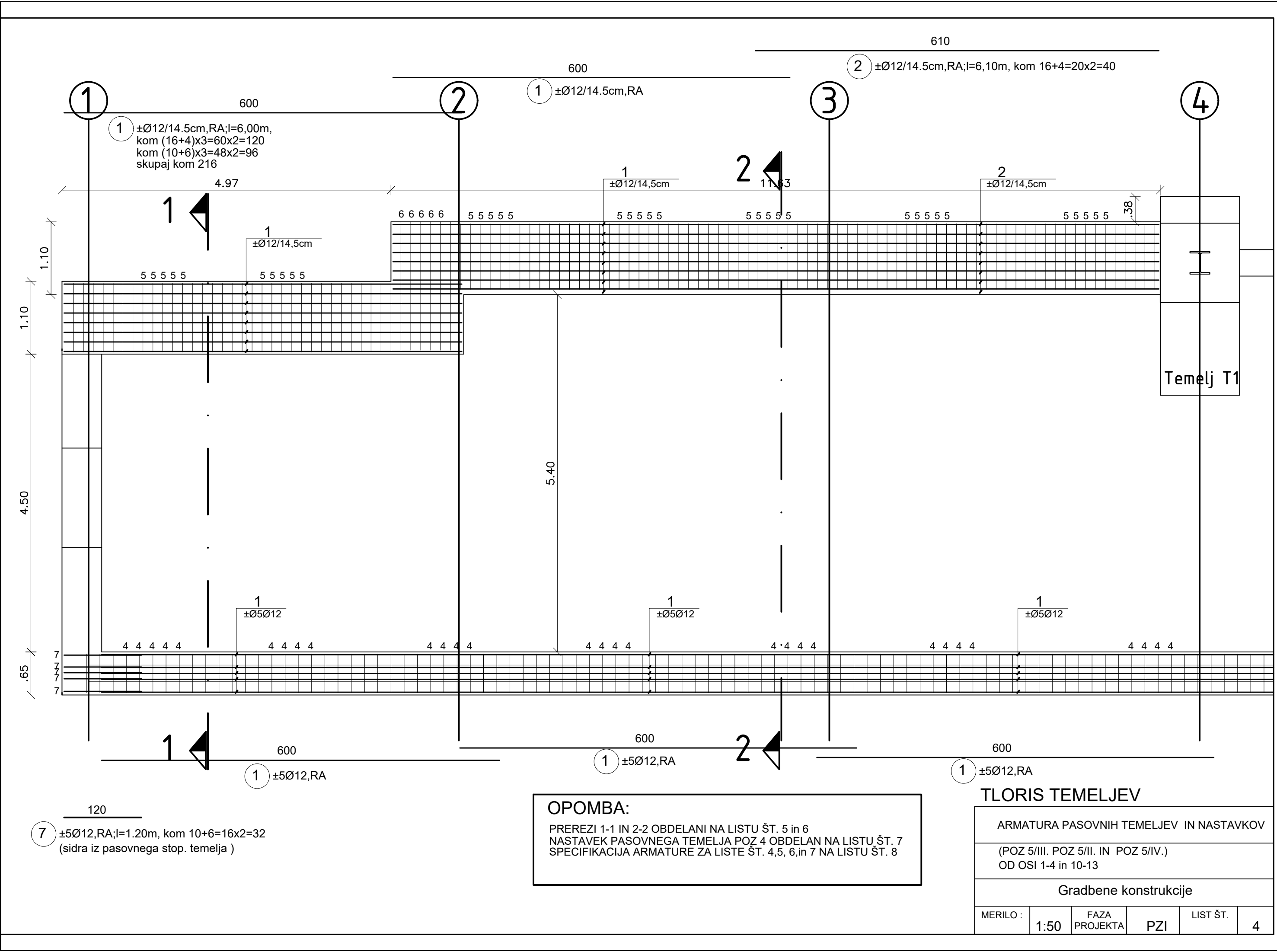
Gradbene konstrukcije

MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT.	2
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SPECIFIKACIJA ARMATURE (IZ LISTA ŠT. 2)

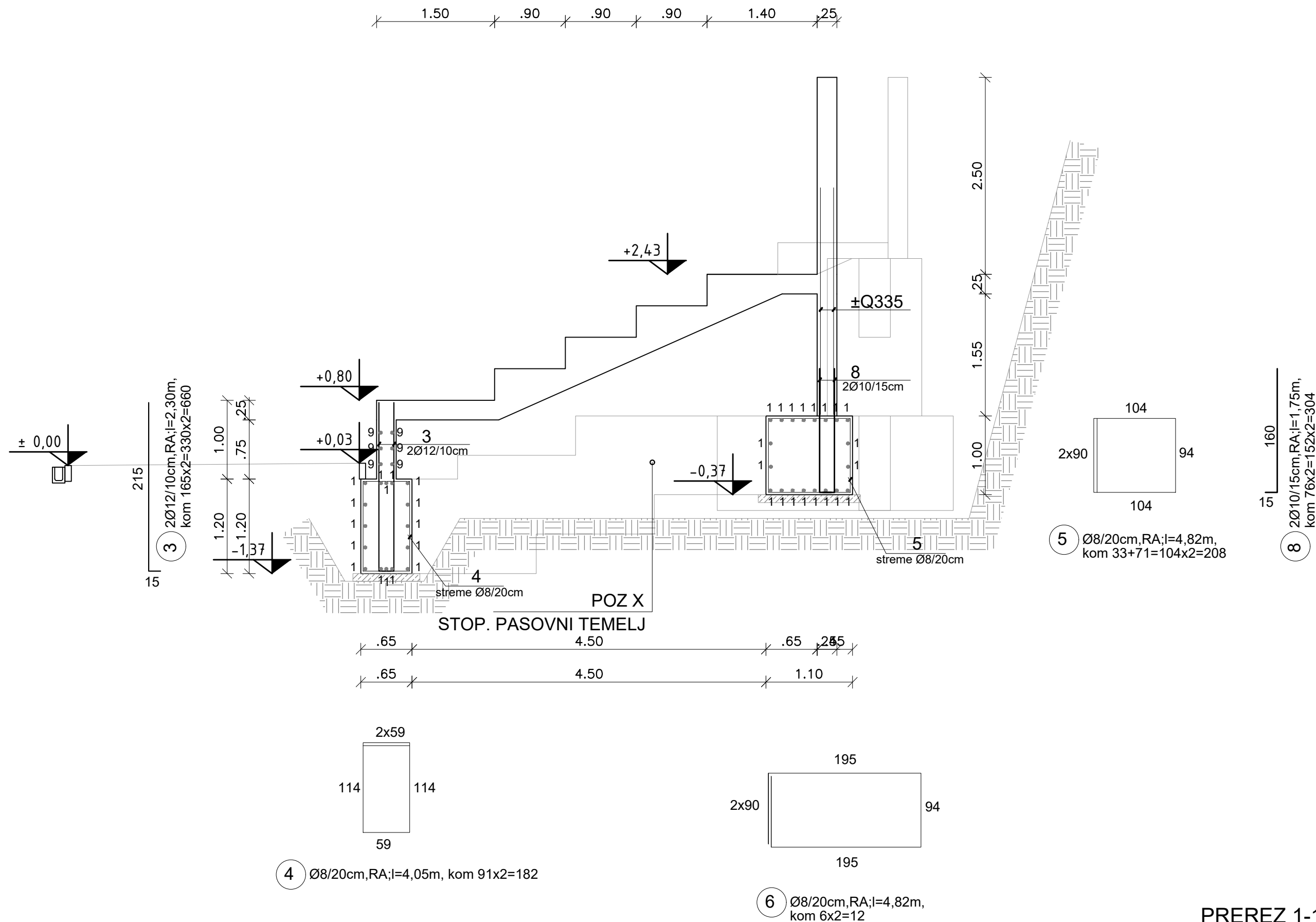
ARMATURA STOP. PASOVNEGA TEMELJA IN
ZAPORNEGA STRANSKEGA ZIDU TRIBUN POZ I.

Poz	Kos	Ø (mm)	L (m)	Kotiranje - oblika krivljenja (ni v merilu)	Skupna dolžina	Teža (kg/m)	Teža (sum kg)
1	32	12	1,20		38.40	0,920	35.33
2	12	12	3,46		41.52	0,920	38.20
3	24	12	2,10		50.40	0,920	46.37
4	24	12	3,00		72.00	0,920	66.24
5	4	12	3,65		14.60	0,920	13.43
6	12	12	1,80		21.60	0,920	19.87
7	12	12	3,15		37.80	0,920	34.78
8	4	12	2,40		9.60	0,920	8.83
9	36	10	1,70		61.20	0,638	39.05
10	20	10	1,90		38.00	0,638	24.24
11	40	10	2,10		84.00	0,638	53.59
12	18	8	4,50		81.00	0,408	33.05
13	20	8	3,50		70.00	0,408	28.56
14	6	8	3,90		23.40	0,408	9.55
						sum kg do Ø12	451.08



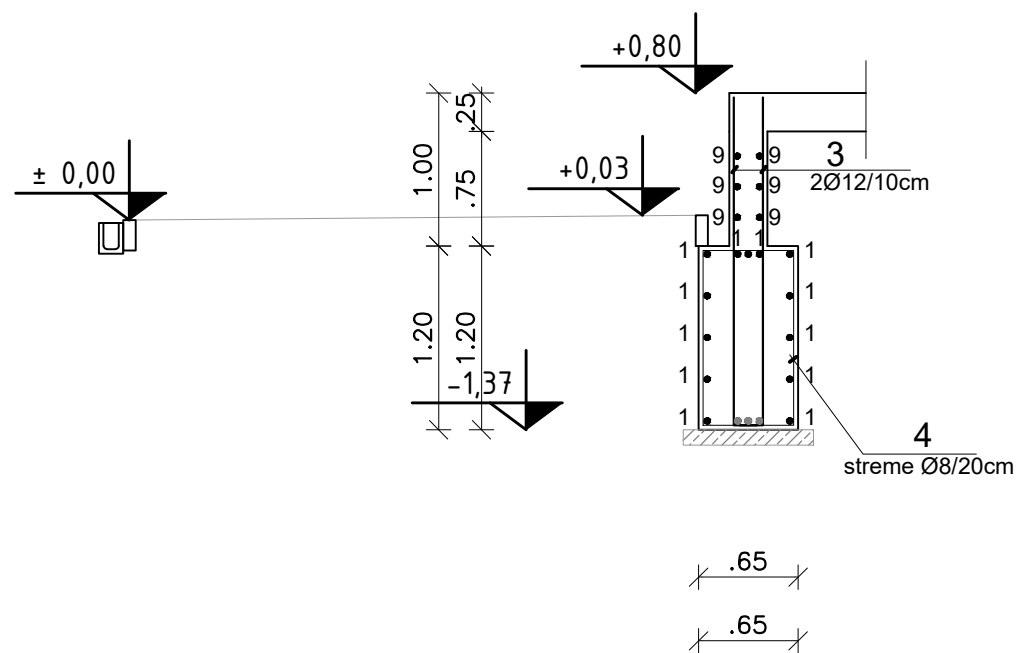
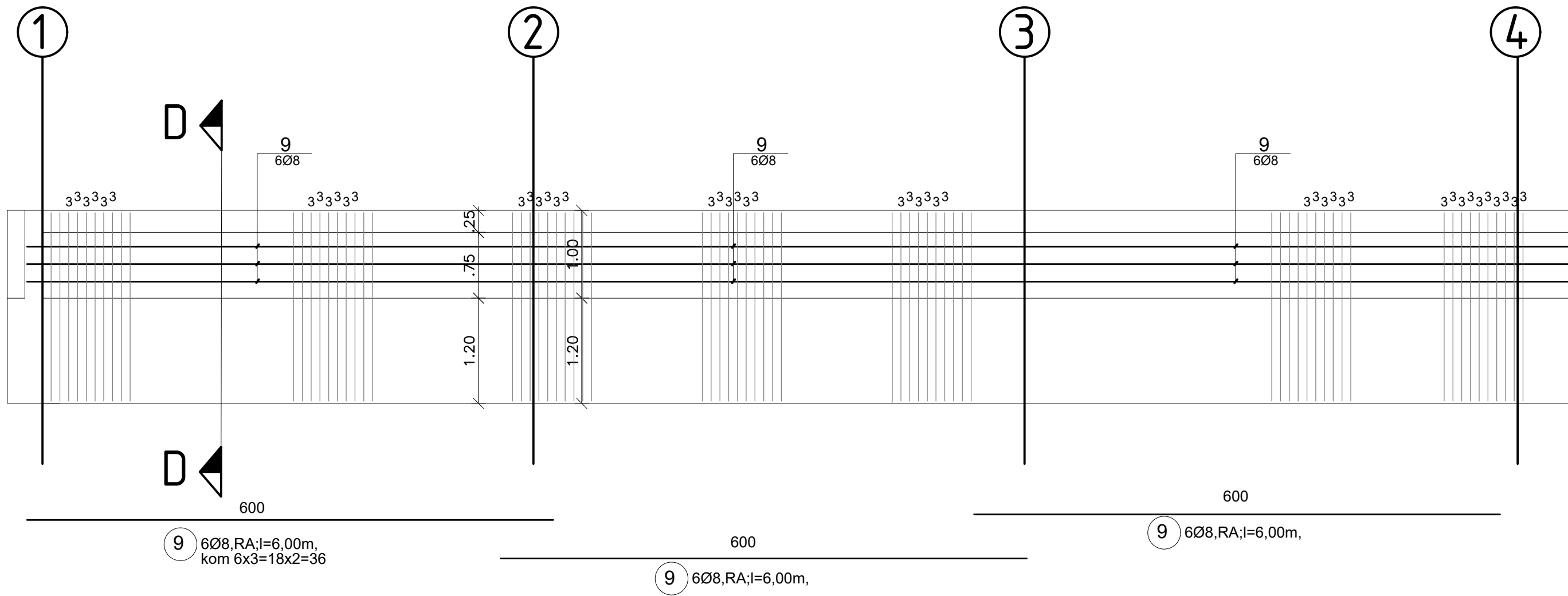
OPOMBA:
PREREZI 1-1 IN 2-2 OBDELANI NA LISTU ŠT. 5 in 6
NASTAVEK PASOVNEGA TEMELJA POZ 4 OBDELAN NA LISTU ŠT. 7
SPECIFIKACIJA ARMATURE ZA LISTE ŠT. 4,5, 6,in 7 NA LISTU ŠT. 8

TLORIS TEMELJEV				
ARMATURA PASOVNIH TEMELJEV IN NASTAVKOV				
(POZ 5/III. POZ 5/II. IN POZ 5/IV.) OD OSI 1-4 in 10-13				
Gradbene konstrukcije				
MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT. 4



PREREZ 1-1

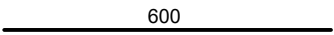
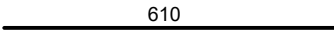
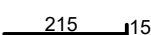
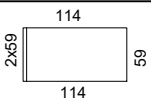
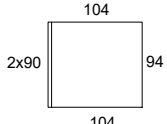
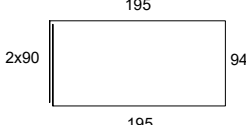
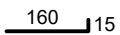
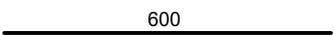
Gradbene konstrukcije				
MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT. 5



NASTAVEK PASOVNEGA TEMELJA POZ 5/IV.
(os 1-3 in 10-13)

Gradbene konstrukcije				
MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT. 7

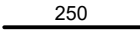
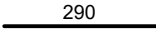
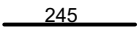
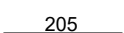
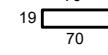
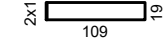
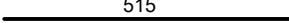
SPECIFIKACIJA ARMATURE (listi št. 4,5,6,7)
 ARMATURA PASOVNIH TEMELJEV IN NASTAVKOV
 (POZ 5/III. POZ 5/II. IN POZ 5/IV.)
 OD OSI 1-4 in 10-13

Poz	Kos	Ø (mm)	L (m)	Kotiranje - oblika krivljenja (ni v merilu)	Skupna dolžina	Teža (kg/m)	Teža (sum kg)
1	216	12	6,00		1296.00	0,408	528.77
2	40	12	6,10		244.00	0,920	224.48
3	660	12	2,30		1518.00	0,920	1396.56
4	182	8	4,05		737.10	0,408	300.74
5	208	8	4,82		1002.56	0,408	409.04
6	12	8	4,82		57.84	0,408	23.60
7	32	12	1,20		38.40	0,920	35.33
8	304	10	1,75		532.00	0,638	339.42
9	36	8	6,00		216.00	0,408	88.13
						sum kg do Ø12	3346.06

SPECIFIKACIJA ARMATURE (listi št. 9)

ARMATURA AB ZIDU POZ 5/V.

OD OSI 1-4 in 10-13

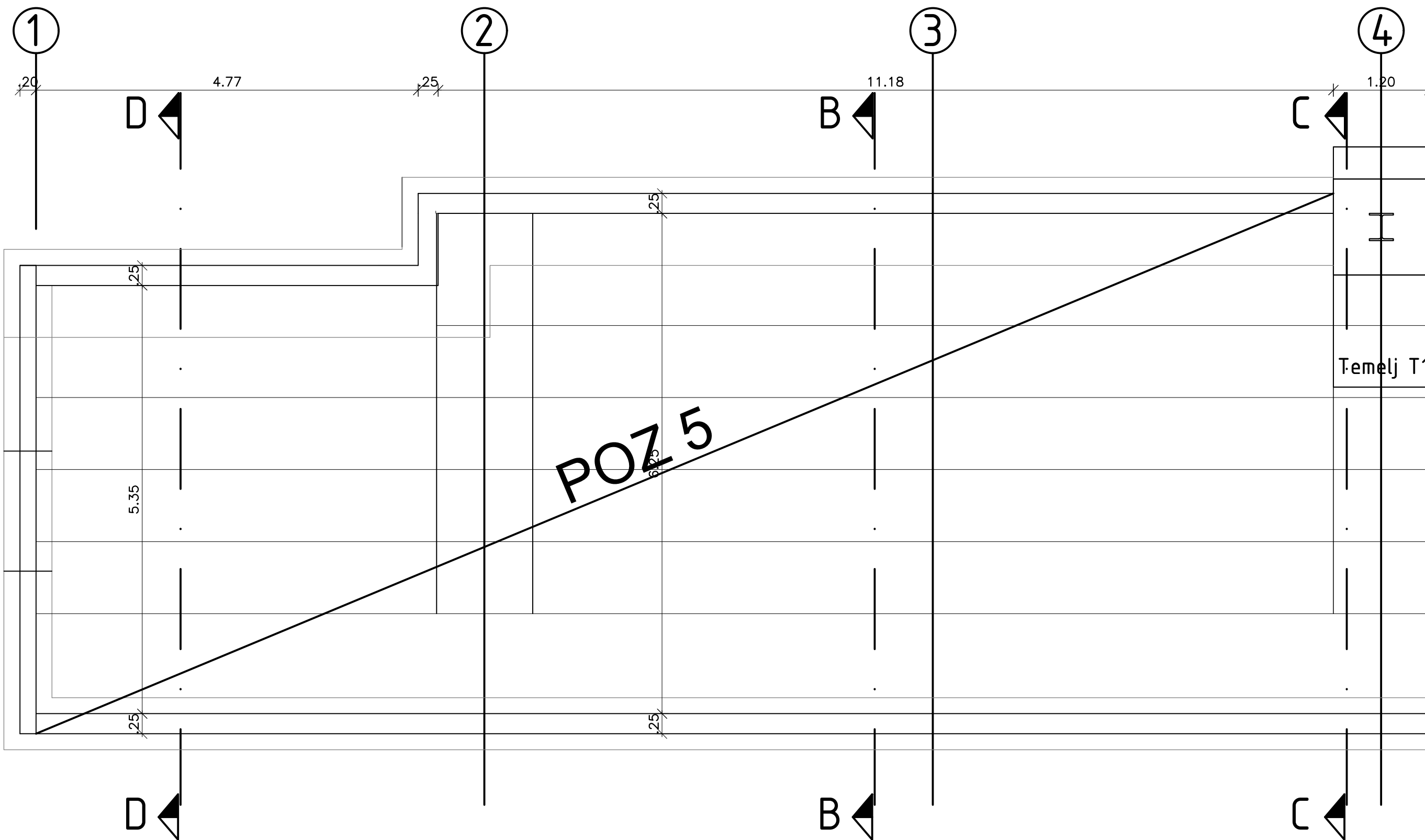
Poz	Kos	Ø (mm)	L (m)	Kotiranje - oblika krivljenja (ni v merilu)	Skupna dolžina	Teža (kg/m)	Teža (sum kg)
1	4	12	2,50		10.00	0,920	9.20
2	32	12	2,90		92.80	0,920	85.38
3	4	12	2,45		9.80	0,920	9.02
4	32	12	2,05		65.60	0,920	60.35
5	54	8	1,19		64.26	0,408	26.22
6	108	8	1,59		171.72	0,408	70.06
7	54	8	2,75		148.50	0,408	60.59
8	40	12	0,90		36.00	0,920	33.12
9	4	10	5,15		20.60	0,638	13.14
10	8	10	6,00		48.00	0,638	30.62
						sum kg doØ12	397.70

SPECIFIKACIJA ARMATURE

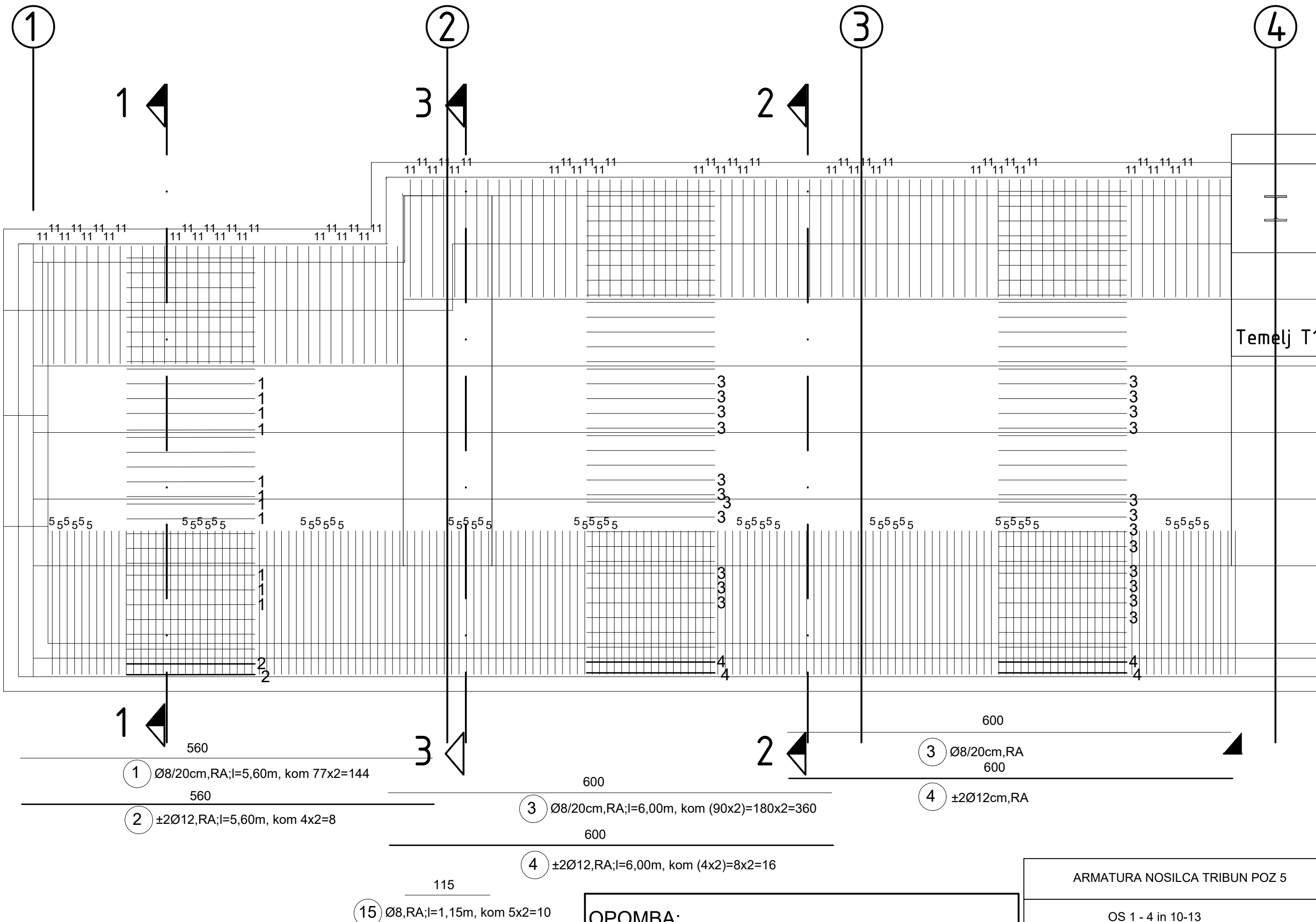
OZN	TIP	a	b	c	d	dim/a	dim/b	kom	kg/m2	sum kg
M1	Q335	150	150	8,0	8,0	2,20	2,50	8	5,33	234.52
M2	Q335	150	150	8,0	8,0	1,25	2,50	4	5,33	66.63
M3	Q335	150	150	8,0	8,0	1,20	2,90	4	5,33	74.19
M4	Q335	150	150	8,0	8,0	2,20	2,90	20	5,33	680.11
M5	Q335	150	150	8,0	8,0	2,05	2,90	4	5,33	126.75
M6	Q335	150	150	8,0	8,0	2,20	2,45	8	5,33	229.83
M7	Q335	150	150	8,0	8,0	1,25	2,45	4	5,33	65.29
M8	Q335	150	150	8,0	8,0	1,20	2,45	4	5,33	62.68
M9	Q335	150	150	8,0	8,0	2,20	2,45	24	5,33	689.49
									Q335	2229.49

št. risbe

10



POZICIJE - TRIBUN					
OS 1 - 4 in 10-13					
Gradbene konstrukcije					
MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT.	11

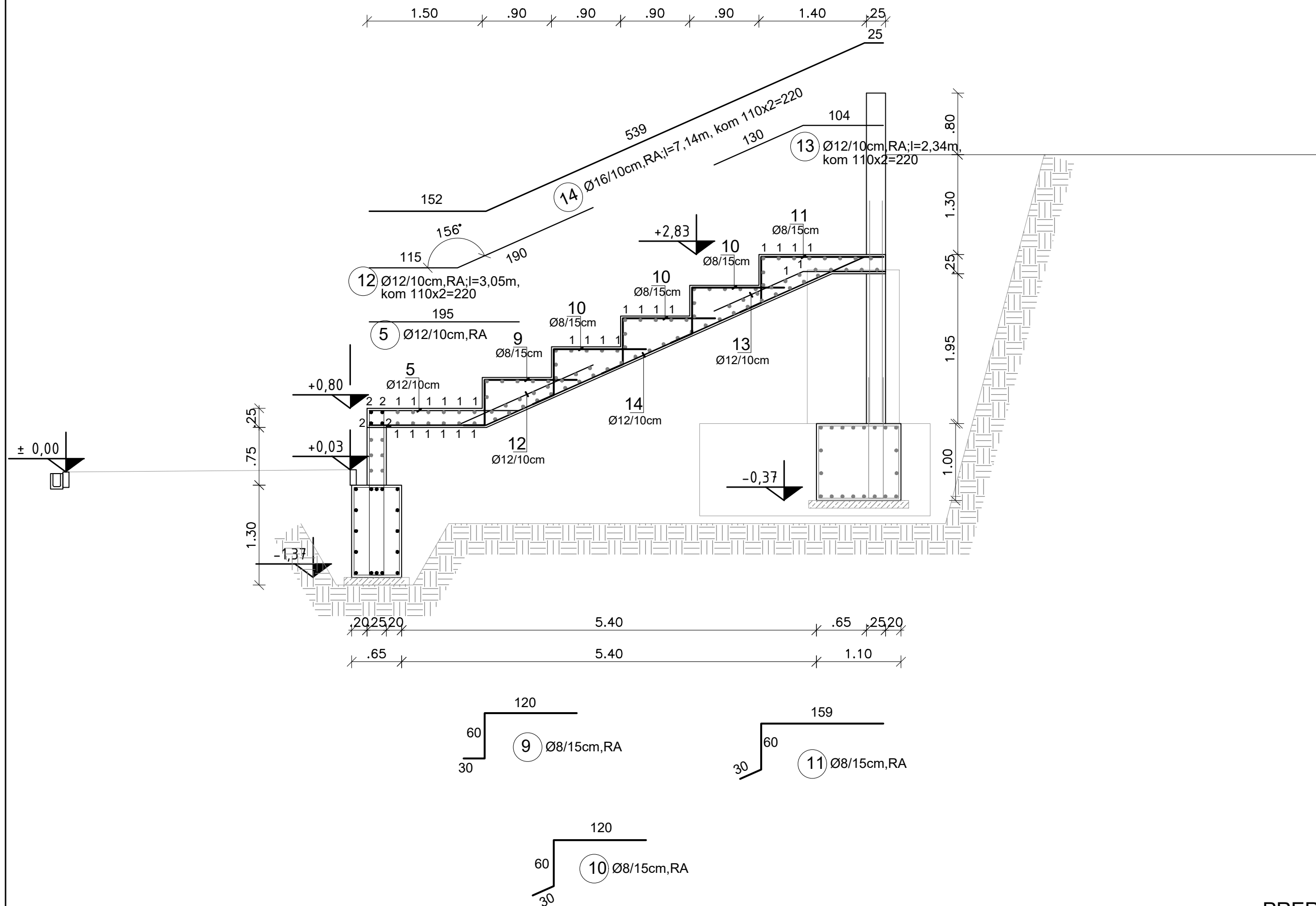


OPOMBA:
PREREZI 1-1, 2-2 IN 3-3 OBDELANI NA LISTU ŠT. 13, 14 in 15
SPECIFIKACIJA ARMATURE ZA LISTE ŠT. 12, 13, 14 in 15 NA LISTU ŠT. 16

ARMATURA NOSILCA TRIBUN POZ 5				
OS 1 - 4 in 10-13				
Gradbene konstrukcije				
MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT. 12



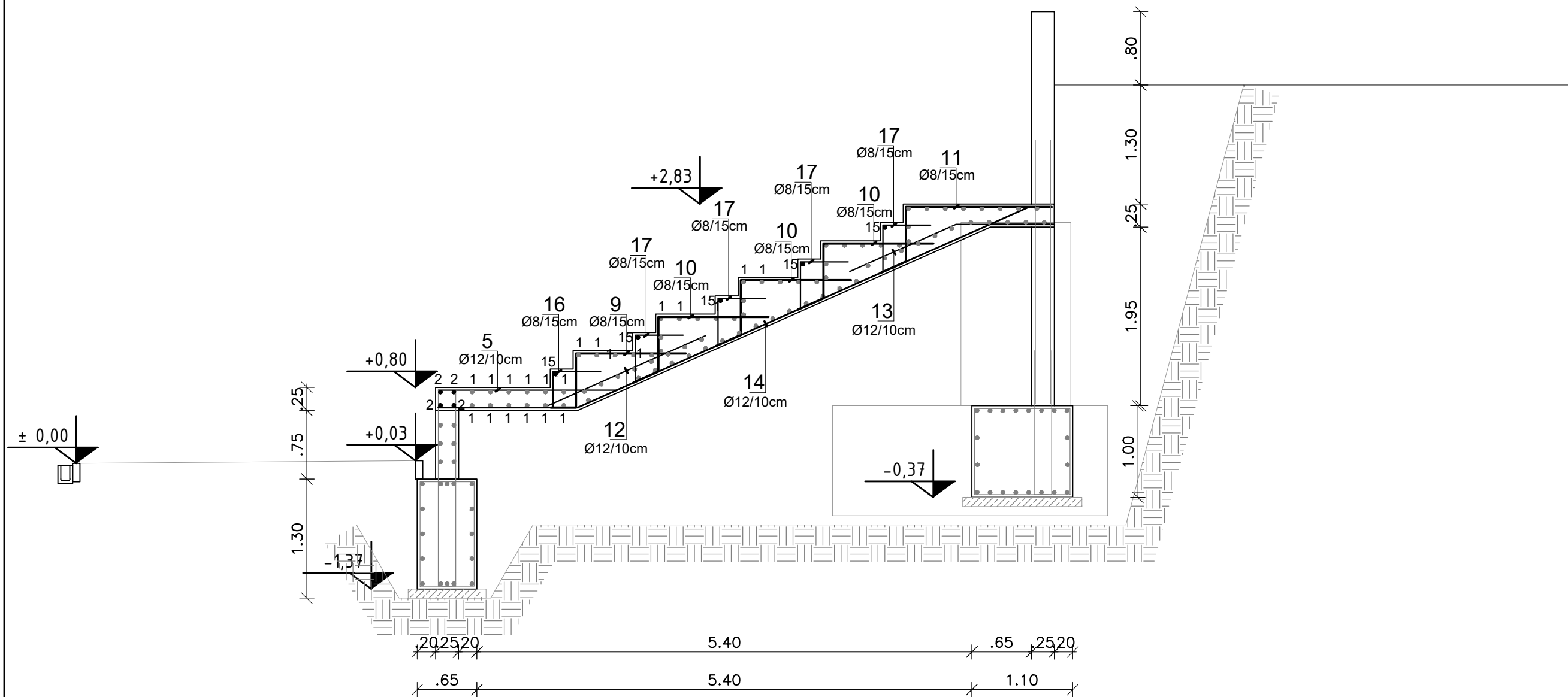
Gradbene konstrukcije					
MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT.	13



PREREZ 2-2

Gradbene konstrukcije

MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT.	14
----------	------	------------------	-----	----------	----



- 52
39
30
- 16 Ø8/15cm, RA; l=1,21m,
kom 8x2=16
- 52
51
30
- 17 Ø8/15cm, RA; l=1,33m,
kom (8x4)=32x2=64

PREREZ 3-3

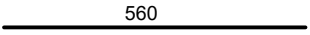
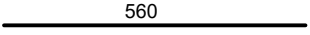
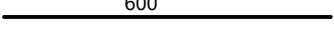
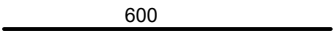
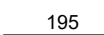
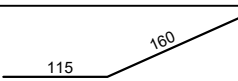
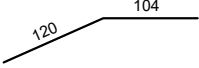
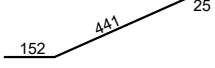
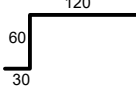
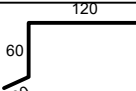
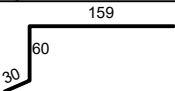
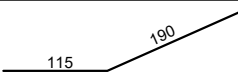
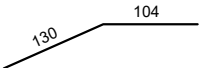
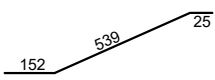
Gradbene konstrukcije

MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT.	15
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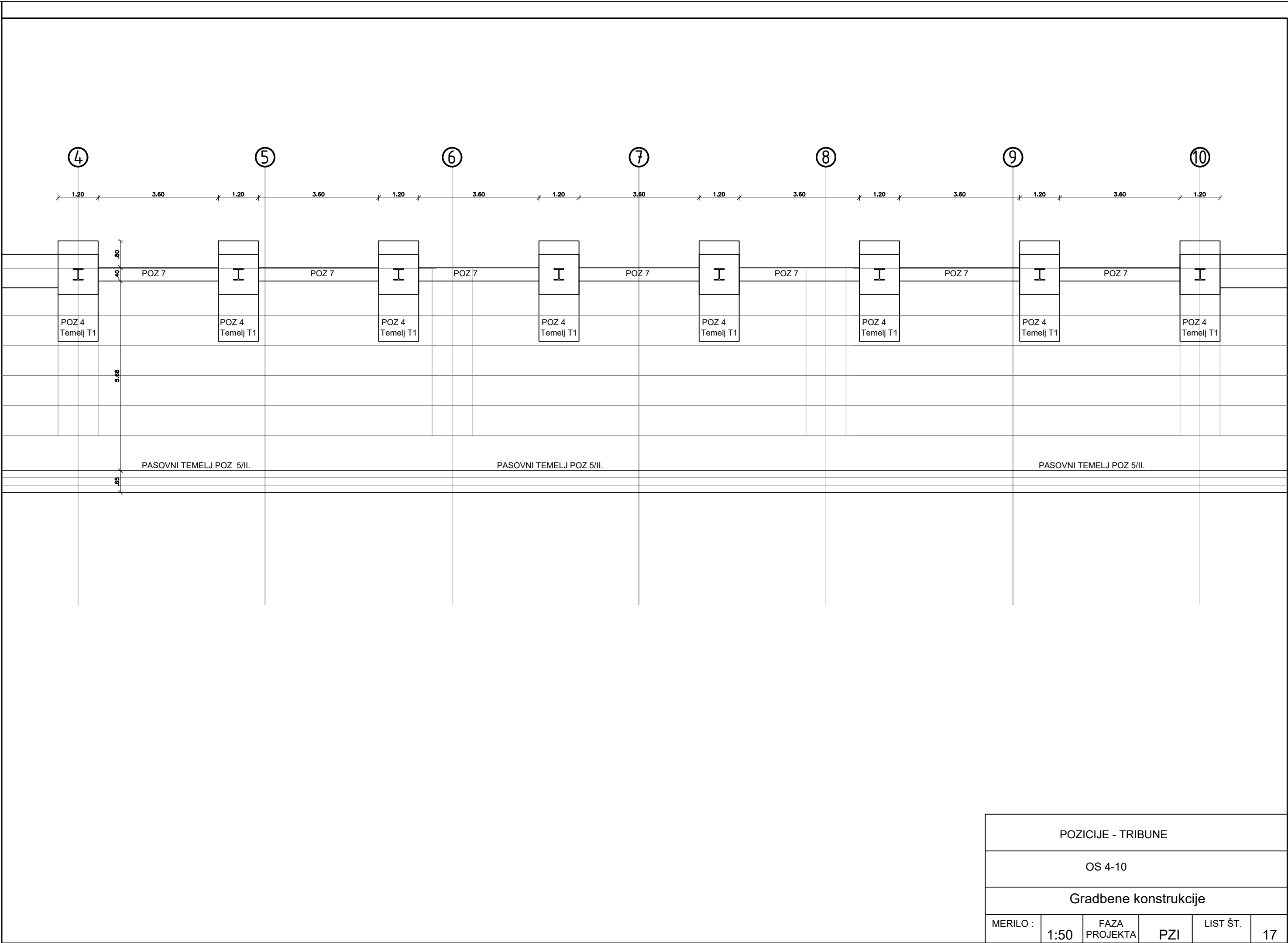
SPECIFIKACIJA ARMATURE (listi št. 12,13,14,15)

ARMATURA NOSILCA TRIBUN POZ 5

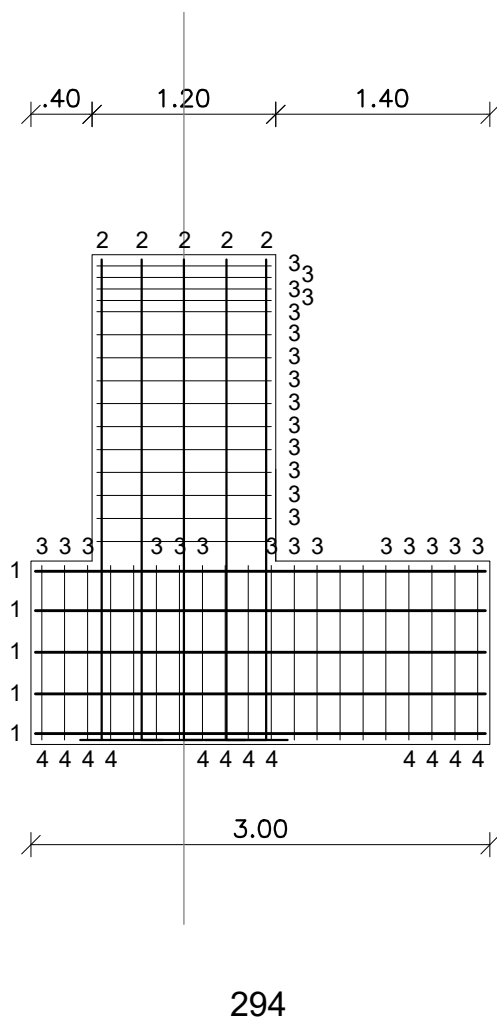
OD OSI 1-4 in 10-13

Poz	Kos	Ø (mm)	L (m)	Kotiranje - oblika krivljenja (ni v merilu)	Skupna dolžina	Teža (kg/m)	Teža (sum kg)
1	144	8	5,60		806.40	0,408	329.01
2	8	12	5,60		44.80	0,920	41.22
3	360	8	6,00		2160.00	0,408	881.28
4	16	12	6,00		96.00	0,920	88.32
5	320	12	1,95		624.00	0,920	574.08
6	100	12	2,75		275.00	0,920	253.00
7	100	12	2,24		224.00	0,920	206.08
8	100	14	6,18		618.00	1,242	767.56
9	216	8	2,10		453.60	0,408	185.07
10	582	8	2,10		1222.20	0,408	498.66
11	216	8	2,49		537.84	0,408	219.44
12	220	12	3,05		671.00	0,920	617.32
13	220	12	2,34		514.80	0,920	473.62
14	220	16	7,14		1570.80	1,621	2546.27
15	10	8	1,15		11.50	0,408	4.69
16	16	8	1,21		19.36	0,408	7.90
17	64	8	1,33		85.12	0,408	34.73
						sum kg doØ12	9506.94
						sum kg nadØ12	3313.82

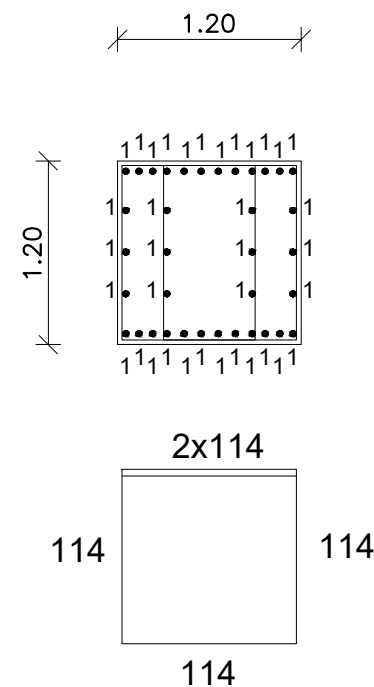
št. risbe



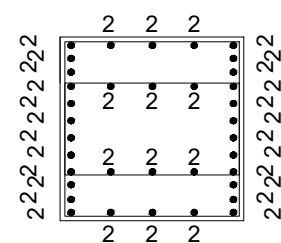
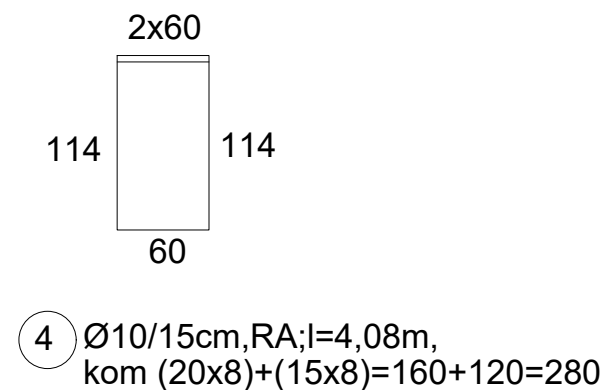
314
40
2 ±12Ø12cm,RA;l=3,54m,
kom 24+12=36x8=288
2.00
1.20



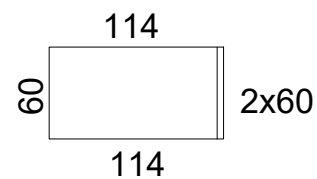
1 ±12Ø14cm,RA;l=2,94m,
kom 24+12=36x8=288



3 Ø10/15cm,RA;l=5,70m,
kom (20x8)+(15x8)=160+120=280



3 Ø10/15;7,5cm,RA



4 Ø10/15;7,5cm,RA

OPOMBA:
ZARADI SIDER IZ TOČKOVNEGA TEMELJA V NOSILEC POZ 6
IN TEMELJNO VEZ POZ 7 JE POTREBNO OBVEZNO ZVRTATI LUKNJE
V OPAŽ
SIDRA OBDELANA NA LISTU ŠT. 20 IN 25

SPECIFIKACIJA ARMATURE NA LISTU ŠT. 19

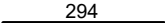
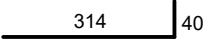
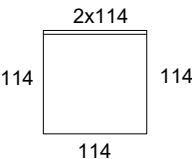
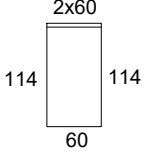
ARMATURA TOČKOVNEGA TEMELJA IN
TEMELJNEGA NASTAVKA POZ 4

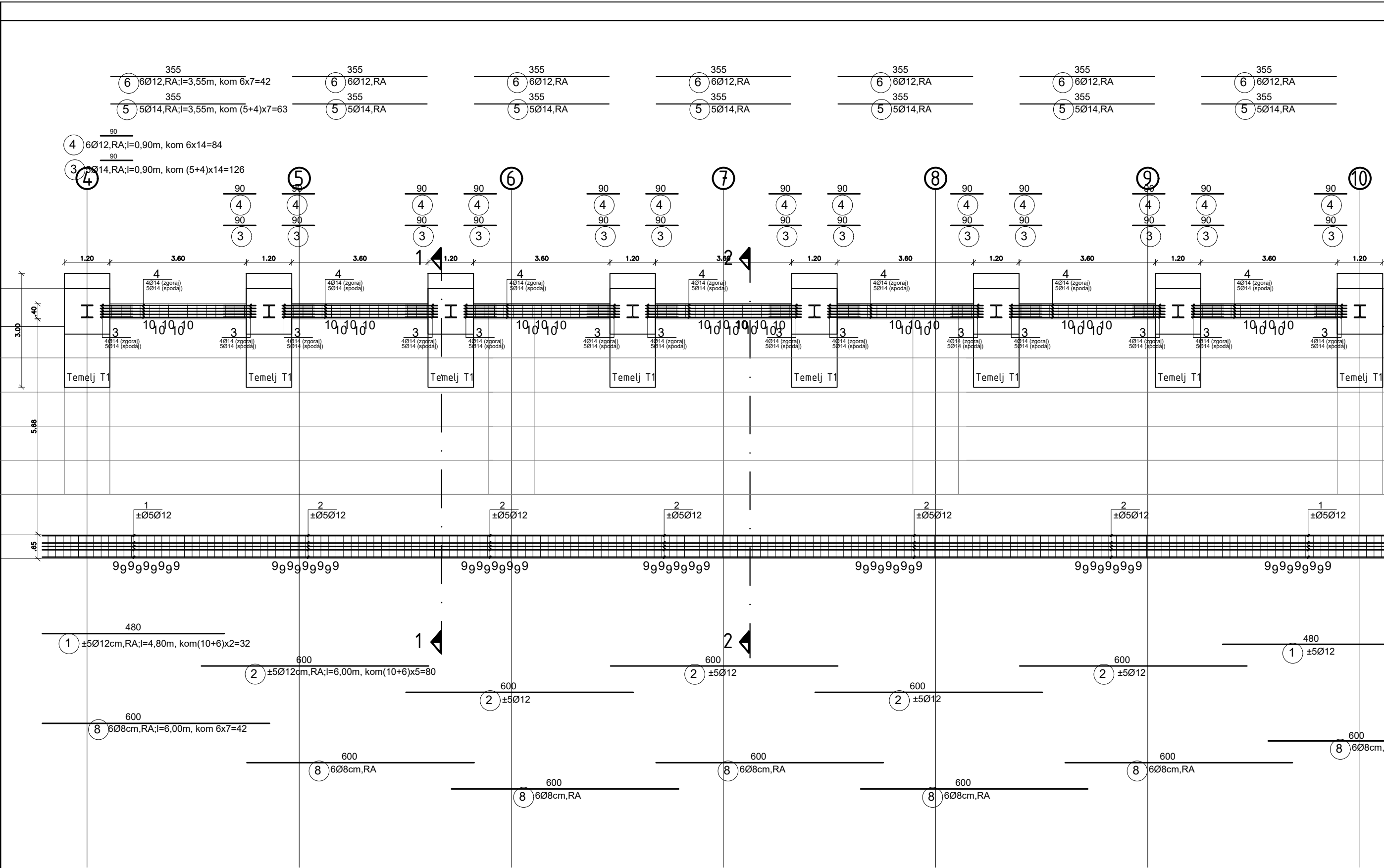
OS 4 - 10 (kom 8)

Gradbene konstrukcije

MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT.	18
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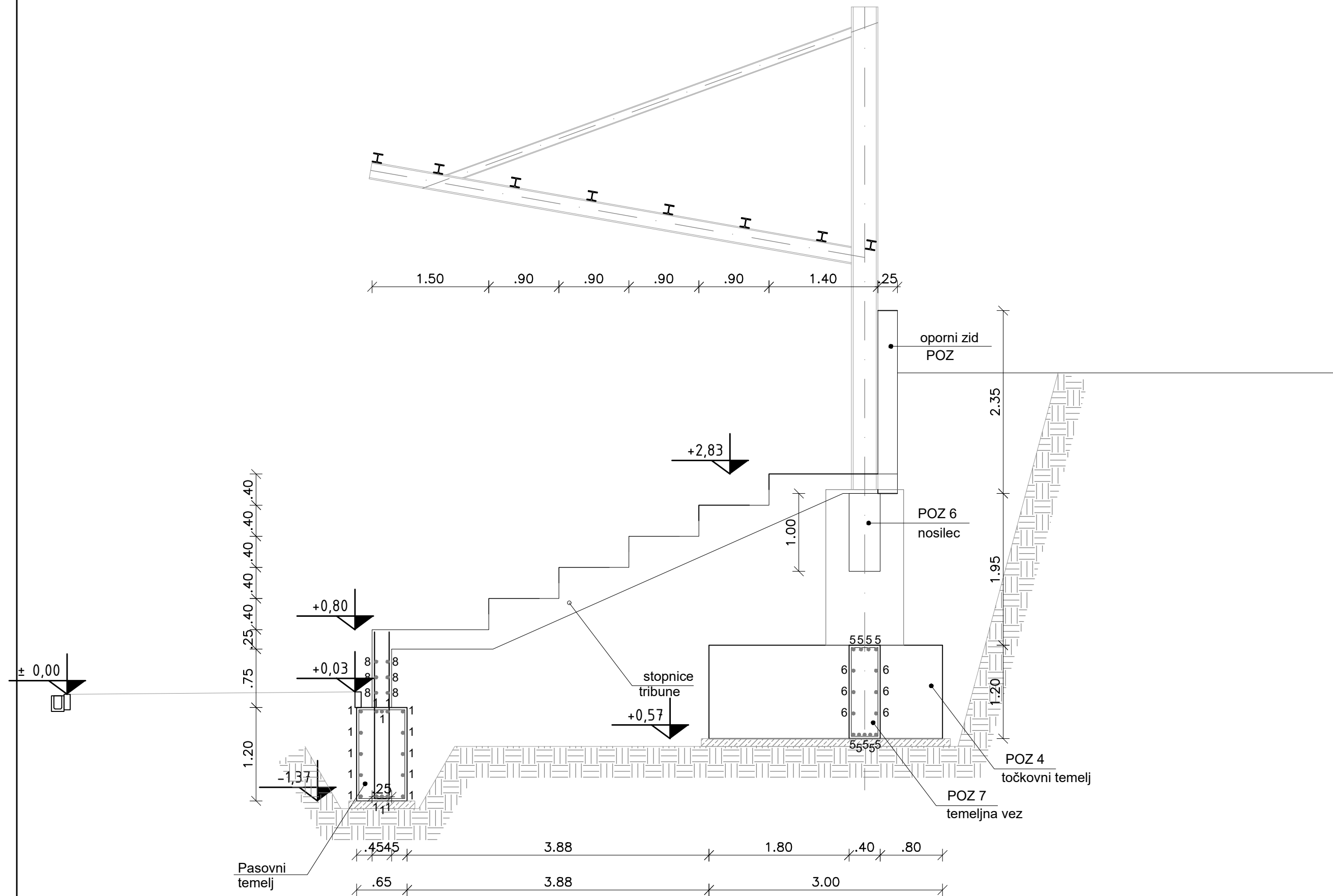
SPECIFIKACIJA ARMATURE IZ LISTA ŠT. 18
ARMATURA TOČKOVNEGA TEMELJA T1 POZ 4

Poz	Kos	Ø (mm)	L (m)	Kotiranje - oblika krivljenja (ni v merilu)	Skupna dolžina	Teža (kg/m)	Teža (sum kg)
1	288	12	2,94		846.72	0,920	778.98
2	288	12	3,54		1019.52	0,920	937.96
3	280	10	5,70		1596.00	0,638	1018.25
4	280	10	4,08		1142.40	0,638	728.85
						sum kg doØ12	3464.04



OPOMBA:
PREREZI 1-1 IN 2-2 OBDELANI NA LISTU ŠT. 21 in 22
SPECIFIKACIJA ARMATURE ZA LISTE ŠT. 20, 21 in 22 NA LISTU ŠT. 23

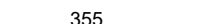
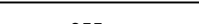
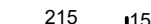
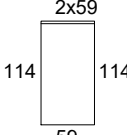
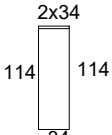
ARMATURA - PASOVNI TEMELJ POZ 5/II.				
ARMATURA - AB NOSILEC POZ 7				
OS 4 - 10				
Gradbene konstrukcije				
MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT. 20

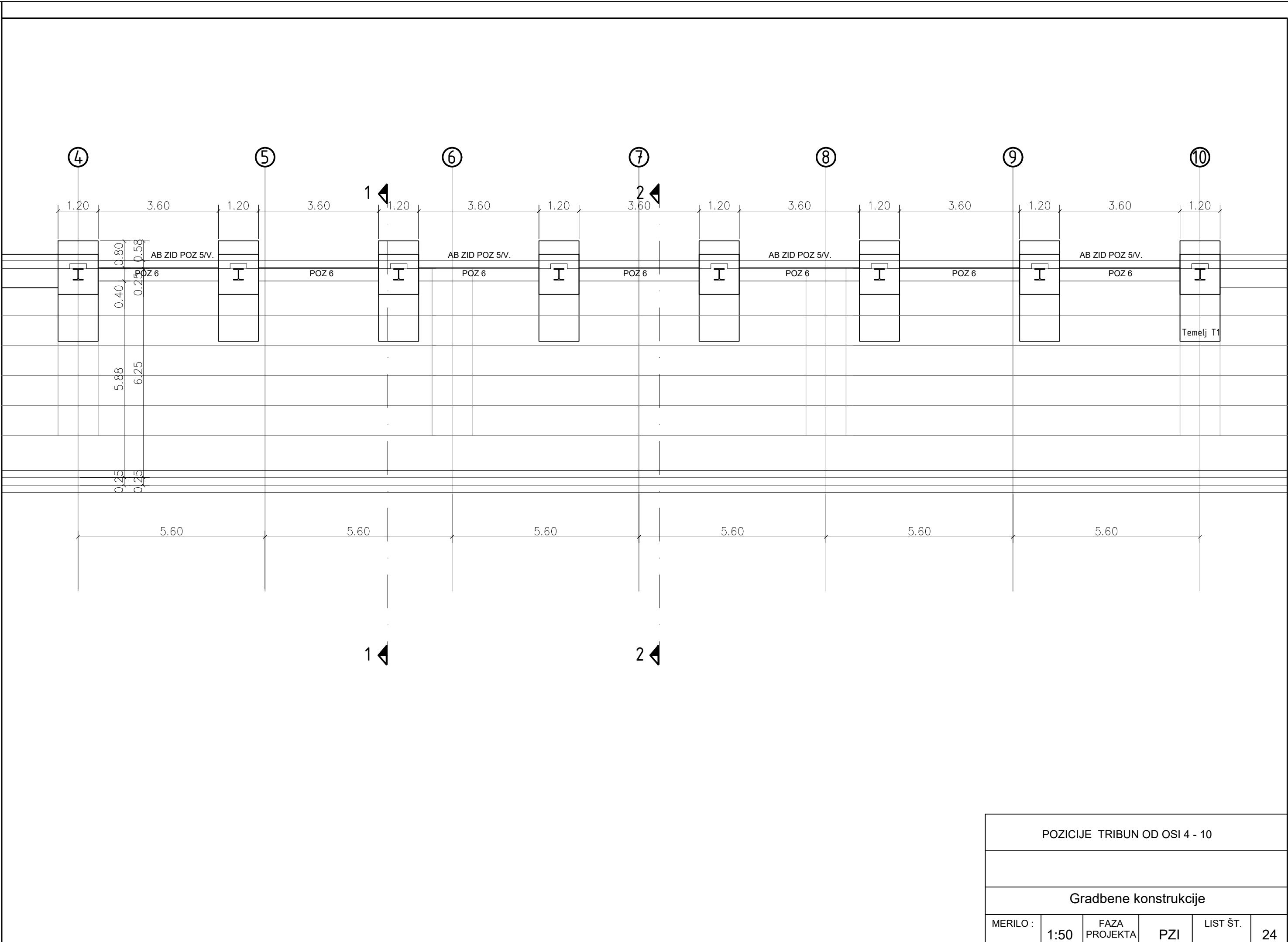


PREREZ 2-2

Gradbene konstrukcije				
MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT. 22

SPECIFIKACIJA ARMATURE (listi št. 20, 21 in 22)
 ARMATURA - PASOVNI TEMELJ POZ 5/II.
 ARMATURA - AB NOSILEC POZ 7
 OS 4 - 10

Poz	Kos	Ø (mm)	L (m)	Kotiranje - oblika krivljenja (ni v merilu)	Skupna dolžina	Teža (kg/m)	Teža (sum kg)
1	32	12	4,80		153.60	0,920	141.31
2	80	12	6,00		480.00	0,920	441.60
3	126	14	0,90		113.40	1,242	140.84
4	84	12	0,90		75.60	0,920	69.55
5	63	14	3,55		223.65	1,242	277.77
6	42	12	3,55		149.10	0,920	137.17
7	720	12	2,30		1656.00	0,920	1523.52
8	42	8	6,00		252.00	0,408	102.82
9	180	8	4,05		729.00	0,408	297.43
10	105	8	3,30		346.50	0,408	141.37
						sum kg doØ12	3410.32
						sum kg nadØ12	418.62



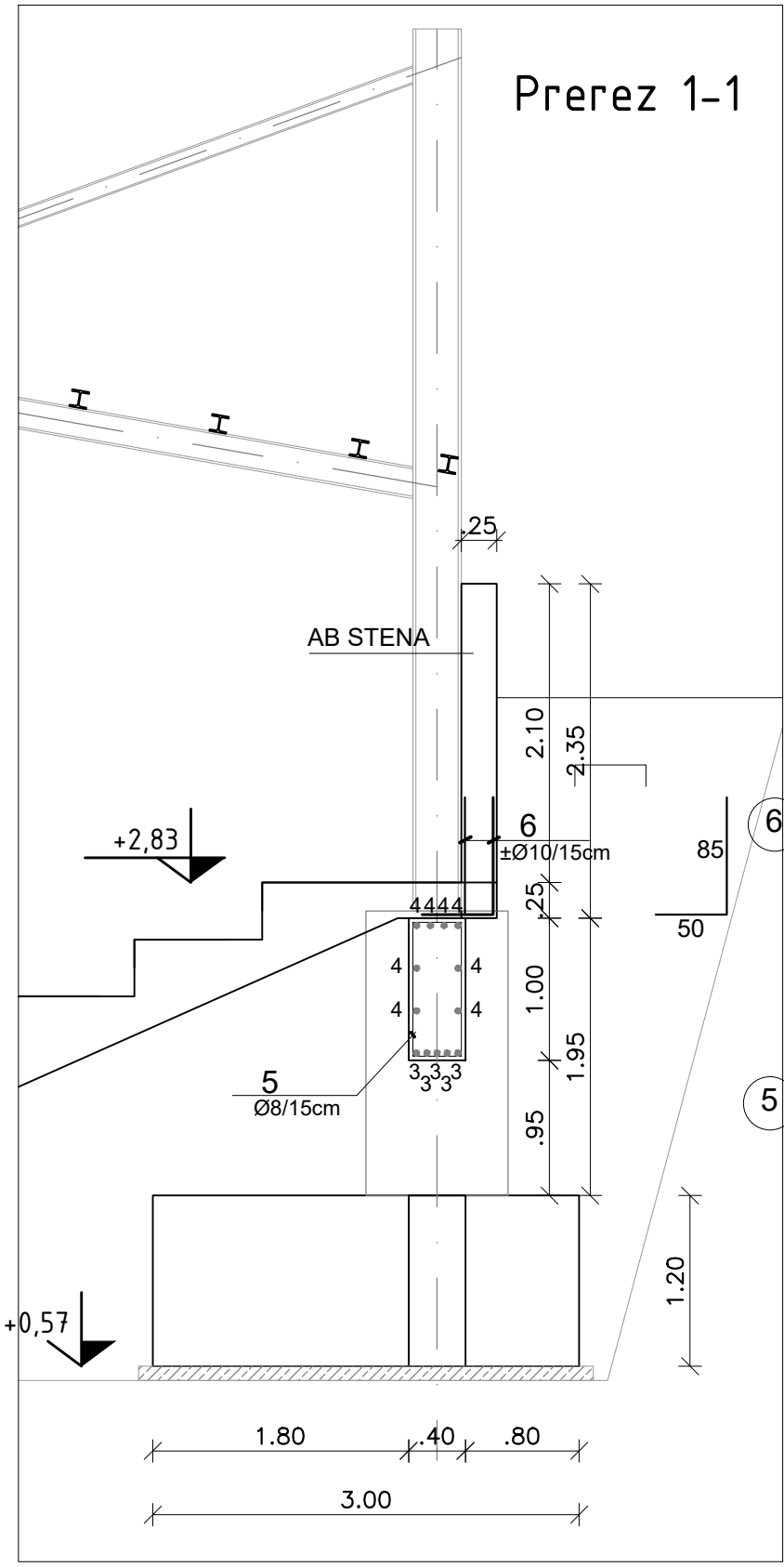
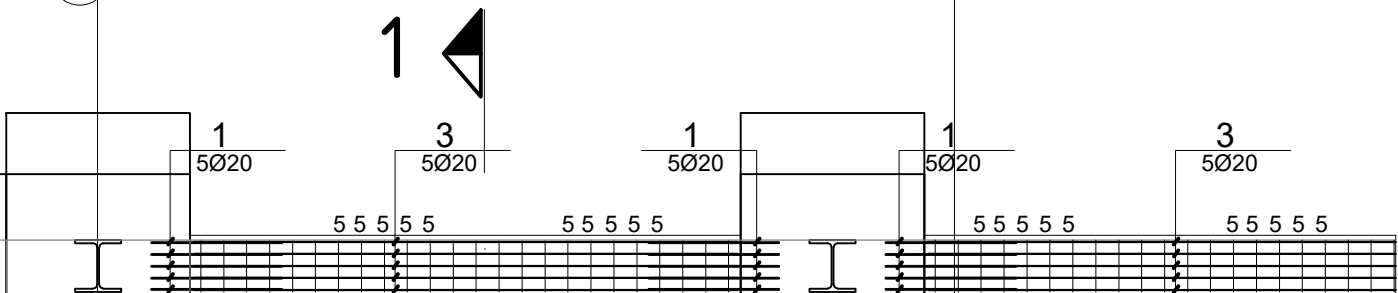
- 355

4 4Ø14,RA;l=3,55m, kom 4+4=8x7=56
- 355

3 5Ø20,RA;l=3,55m, kom 5x7=35
- 90

2 4Ø14,RA;l=0,90m, kom (4+4)x14=112
- 90

1 5Ø20,RA;l=3,55m, kom 5x14=70



- Ø10/15cm,RA;l=1,35m, kom 232x2=464

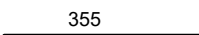
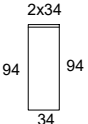
2x34

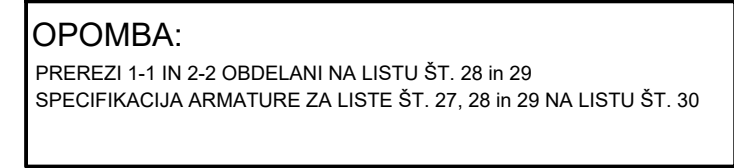
94 94

34
- Ø8/15cm,RA;l=2,90m, kom 24x7=168

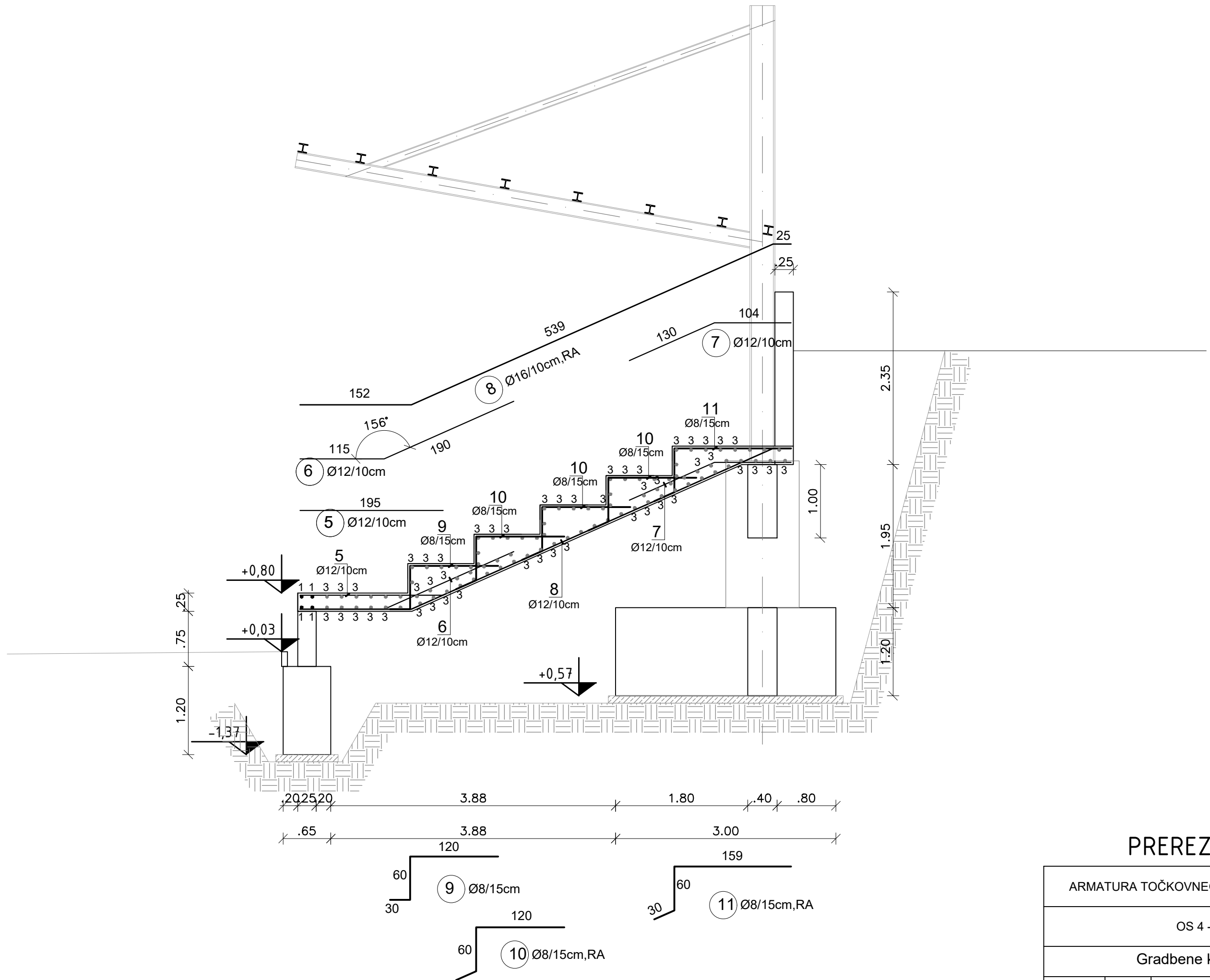
specifikacija armature na listu št. 26					
ARMATURA AB NOSILCA POZ 6					
KOM 7					
Gradbene konstrukcije					
MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT.	25

SPECIFIKACIJA ARMATURE IZ LISTA ŠT. 25
ARMATURA AB NOSILCA POZ 6

Poz	Kos	Ø (mm)	L (m)	Kotiranje - oblika krivljenja (ni v merilu)	Skupna dolžina	Teža (kg/m)	Teža (sum kg)
1	70	20	0,90		63.00	2,470	155.61
2	112	14	0,90		100.80	1,242	125.19
3	35	20	3,55		124.25	2,470	306.90
4	56	14	3,55		198.80	1,242	246.91
5	105	8	2,90		304.50	0,408	124.24
6	105	10	1,35		141.75	0,638	90.44
						sum kg doØ12	214.67
						sum kg nadØ12	834.61



ARMATURA NOSILCA TRIBUN POZ 5					
OD OSI 4 - 10					
Gradbene konstrukcije					
MERILO :	1:100	FAZA PROJEKTA	PZI	LIST ŠT.	27



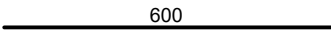
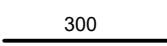
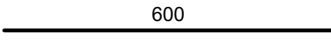
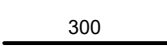
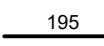
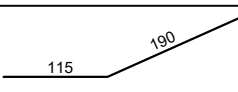
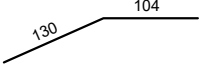
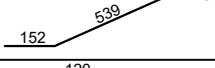
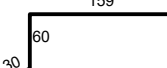
PREREZ 2-2

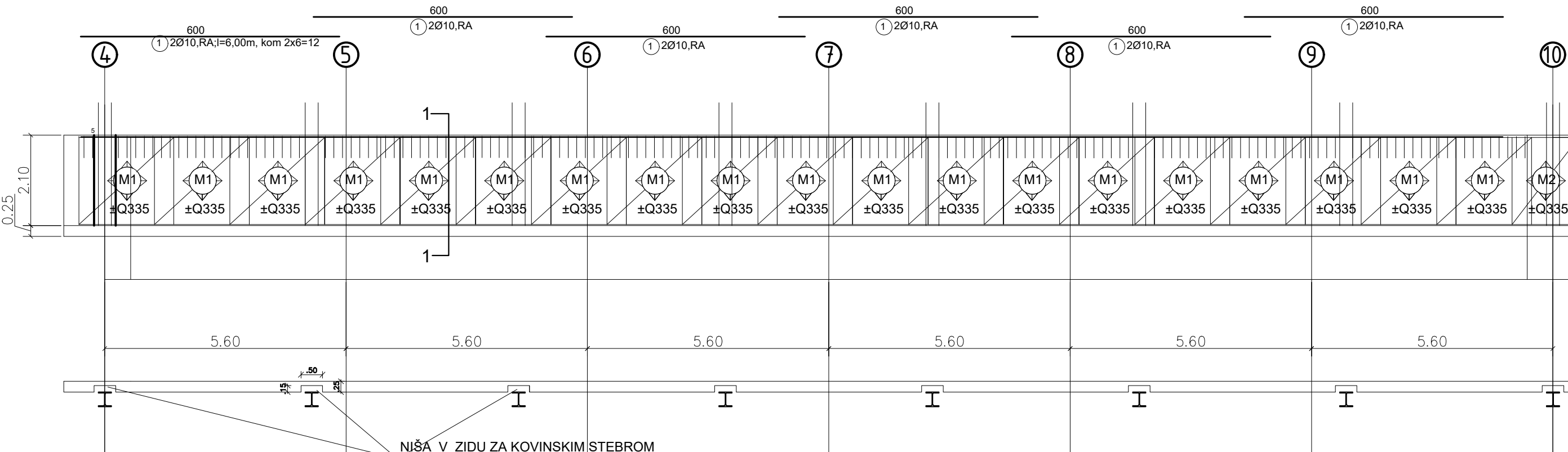
ARMATURA TOČKOVNEGA TEMELJA TRIBUN					
OS 4 - 10 (kom 8)					
Gradbene konstrukcije					
MERILO :	1:50	FAZA PROJEKTA	PZI	LIST ŠT.	29

SPECIFIKACIJA ARMATURE (listi št. 27,28,29)

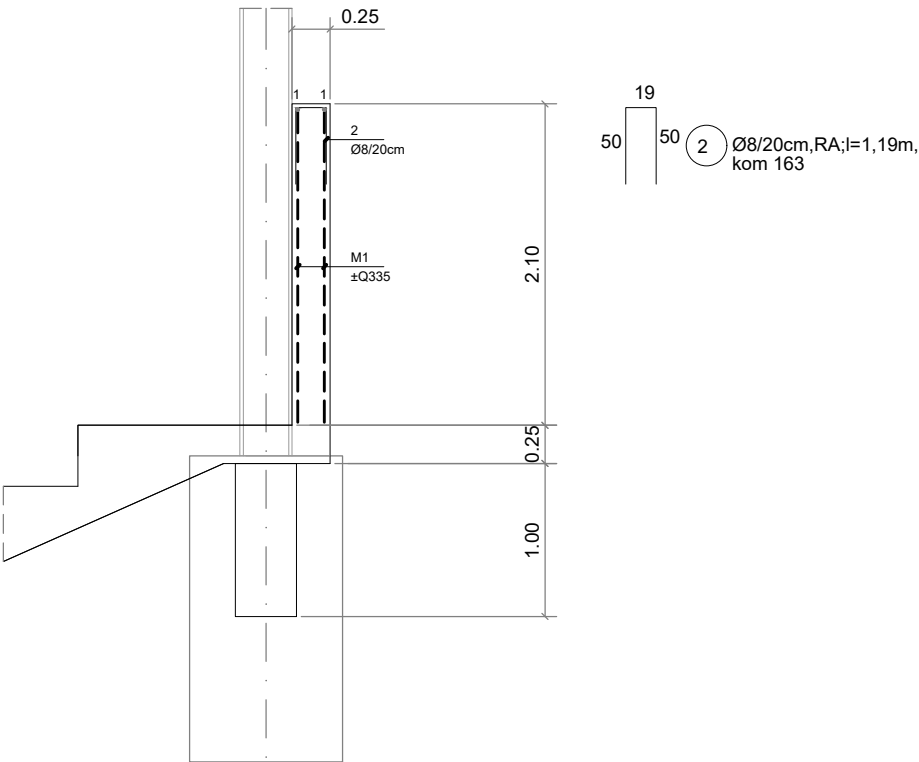
ARMATURA NOSILCA TRIBUN POZ 5

OD OSI 4 - 10

Poz	Kos	Ø (mm)	L (m)	Kotiranje - oblika krivljenja (ni v merilu)	Skupna dolžina	Teža (kg/m)	Teža (sum kg)
1	24	12	6,00		144.00	0,920	132.48
2	4	12	3,00		12.00	0,920	11.04
3	540	8	6,00		3240.00	0,408	1321.92
4	90	8	3,00		270.00	0,408	110.16
5	347	12	1,95		676.65	0,920	622.52
6	347	12	3,05		1058.35	0,920	973.68
7	347	12	2,34		811.98	0,920	747.02
8	347	16	7,14		2477.58	1,621	4016.16
9	232	8	2,10		487.20	0,408	198.78
10	696	8	2,10		1461.60	0,408	596.33
11	232	8	2,49		577.68	0,408	235.69
12	32	8	1,21		38.72	0,408	15.80
13	128	8	1,33		170.24	0,408	69.46
14	20	8	1,15		23.00	0,408	9.38
						sum kg doØ12	5044.27
						sum kg nadØ12	4016.16



Prerez 1-1



Poz	Kos	Ø (mm)	L (m)	Kotiranje - oblika krivljenja (ni v merilu)	Skupna dolžina	Teža (kg/m)	Teža (sum kg)
1	12	10	6,00	600	72.00	0,638	45.94
2	163	8	1,19	50 19 50	193.97	0,408	79.14
						sum kg doØ12	125.08

SPECIFIKACIJA ARMATURE										
OZN	TIP	a	b	c	d	dim/a	dim/b	kom	kg/m2	sum kg
M1	Q335	150	150	8,0	8,0	2,20	2,05	38	5,33	913.46
M2	Q335	150	150	8,0	8,0	1,55	2,05	2	5,33	33.87
									Q335	947.33

ARMATURA AB ZIDU POZ 5					
OD OSI 4 - 10					
Gradbene konstrukcije					
MERILO :	1:100	FAZA PROJEKTA	PZI	LIST ŠT.	31